

Traffic Impact Study

Proposed Residential Subdivision with Commercial

Towncentre Dr, Belleville



March 18, 2024
Project No 2317-23

Project N° 2317-23

Asurza Engineers Ltd.

7078 Liberty St. North

Bowmanville, ON

L1C 6K4

P: 905.263.4399

www.asurza.ca

March 18, 2024

Mr. George Taylor, Planner
Ministry of Transportation, Ontario
1355 John Counter Blvd
Kingston, ON
K7L 5A3

Reference: Towncentre Dr – Residential Subdivision with Commercial
Belleville, ON
Traffic Impact Study Report
Project N° 2317-23

Dear Mr. Taylor,

Asurza Engineers Ltd. is pleased to submit the enclosed Traffic Impact Study in support of the proposed residential subdivision with commercial to be located in proximity to Highway 62 in northern Belleville. The study was prepared on behalf of the proponent as part of the documentation required by the City of Belleville and The Ministry of Transportation Ontario.

Should you have any questions regarding to this report, please do not hesitate to contact the undersigned.


Martin Asurza, M. Eng., P. Eng.
Senior Transportation/Traffic Engineer
martin@asurza.ca



LICENSED PROFESSIONAL ENGINEER
Mar 18, 2024
M.C. ASURZA AYVAR
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Project Team Member List

Project Manager: Martin C. Asurza, M.Eng., P.Eng.

Project Technical Staff: Luca Di Placido, B.Eng.

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Executive Summary

Asurza Engineers Ltd. was retained to undertake a traffic impact study in order to review, assess, and determine any traffic impact the proposed development may generate on the adjacent roads and intersections.

The subject site is located adjacent to Highway 62 in Northern Belleville, bounded between a recent subdivision on the west and surrounded by an established commercial area. Currently a vacant land of approx. 8 hectares, the development is planned to include single family attached & detached houses, low-rise multifamily housing, and a low-rise residential building with ground-floor commercial space.

It should be noted that the proposed land uses in this report are as defined in the Trip Generation Manual, a publication by the Institute of Transportation Engineers (not the city's zoning by-law).

Traffic movement counts undertaken on May 10, 2022, were provided by MTO for the intersections of Hwy 62 at Farnham Rd, Roy Blvd, and Maitland Dr. A growth rate of 1.5% was used to project the counts to 2024. A video-based count was performed by Asurza Engineers on January 18, 2024, to gain data for Sidney St / Kempton Ave. In addition, the traffic signal timings at Hwy 62 / Farnham Rd and Hwy 62 / Maitland Dr were provided by MTO.

An intersection capacity analysis was undertaken for all relevant scenarios within the Synchro 11 software, based on the Highway Capacity Manual (HCM) methodology.

For the existing conditions (2024), results show that the intersections are currently operating with acceptable control delays during peak hours. Almost all movements have an LOS "C" or better, and all movements are showing v/c ratios well below 0.85.

In order to establish base conditions for the comparison and evaluation of future scenarios, a 1.5% growth rate was used to project the traffic volumes throughout the study years.

Also, to maintain accuracy in the analysis, the ongoing build-out of the nearby "Settler's Ridge East Phase 2" subdivision should be accounted for. As of the date of this report, the subdivision has finished its Phase 1 build-out; therefore, the new trips resulting from Phase 2 (36 single detached and 31 townhome units) have been estimated and distributed to the Sidney St / Kempton Ave intersection.

The results show that, without the proposed development, delays will moderately increase over time as expected. Throughout the study years, all movements will continue to operate with acceptable levels of service.

The estimation of trips generated by the proposed development were derived from the Trip Generation Manual, 11th Edition. The proposed development is estimated to generate 112 and 137 new trips during the AM and PM peak hours, respectively. The vast majority of new trips generated are assumed to be entering/exiting the development using Hwy 62 / Roy Blvd and are distributed according to the existing traffic patterns.

The future total traffic volumes are obtained by adding the background traffic volumes to the total new traffic volumes generated by the proposed development.

The results show that, including the proposed development, the delays and LOS for all movements remain basically the same as the background scenario. The most critical movements are expected to be the westbound-left and southbound-thru movements at Highway 62 / Maitland Dr, which both show LOS "D" by 2032 regardless of the proposed development. Also, the v/c ratio for all movements will remain well below 0.85 throughout the study years.

From the traffic point of view, it is concluded that the proposed residential development can occur without any impacts to traffic operations.

Table of Contents

1	Introduction	1
1.1	Overview	1
1.2	Objectives.....	1
2	Existing Conditions	2
2.1	Study Area	2
2.2	The Site.....	3
2.3	Existing Roadway Network	4
2.4	Traffic Data	5
2.5	Existing Traffic Operations	9
3	Background Traffic Volumes and Operations	11
3.1	Background Traffic Volumes	11
3.2	Background Traffic Operations	16
4	Proposed Development Traffic Forecasting	18
4.1	Traffic Impact Study Methodology	18
4.2	Site Trip Generation	18
4.3	Trip Distribution/Assignment	20
5	Future Traffic Volumes and Operations	25
5.1	Future Total Traffic Volumes	25
5.2	Future Total Traffic Operations	30
6	Auxiliary Lane Review	33
7	Conclusions/Recommendations	34

List of Exhibits

Exhibit 1: Study Area.

Exhibit 2: The Site.

Exhibit 3: Existing Lane Configuration at Intersections.

Exhibit 4: Existing AM Peak Hour Volumes, 2024.

Exhibit 5: Existing PM Peak Hour Volumes, 2024.

Exhibit 6: Background AM Peak Hour Volumes, 2027.

Exhibit 7: Background PM Peak Hour Volumes, 2027.

Exhibit 8: Background AM Peak Hour Volumes, 2032.

Exhibit 9: Background PM Peak Hour Volumes, 2032.

Exhibit 10: Trip Distribution, AM Peak Hour.

Exhibit 11: Trip Distribution, PM Peak Hour.

Exhibit 12: New Trips Generated, AM Peak Hour, 2027.

Exhibit 13: New Trips Generated, PM Peak Hour, 2027.

Exhibit 14: Total AM Peak Hour Volumes, 2027.

Exhibit 15: Total PM Peak Hour Volumes, 2027.

Exhibit 16: Total AM Peak Hour Volumes, 2032.

Exhibit 17: Total PM Peak Hour Volumes, 2032.

List of Tables

Table 1: Level of Service Definition.

Table 2: Intersection Capacity, Existing Volumes, 2024.

Table 3: Intersection Capacity, Background Volumes, 2027.

Table 4: Intersection Capacity, Background Volumes, 2032.

Table 5: ITE Trip Rates and Dir. Distributions per Land Use.

Table 6: Estimation of Trips Generated by the Proposed Development.

Table 7: Intersection Capacity, Total Volumes, 2027.

Table 8: Intersection Capacity, Total Volumes, 2032.

Appendices

Appendix A – Preliminary Concept Plan and Draft Plan of Subdivision

Appendix B – Traffic Data

Appendix C – Existing Signal Timings

Appendix D – Traffic Volume Projections

Appendix E – Synchro Reports, Existing Volumes 2024

Appendix F – Synchro Reports, Background Volumes 2027

Appendix G – Synchro Reports, Background Volumes 2032

Appendix H – Synchro Reports, Total Volumes 2027

Appendix I – Synchro Reports, Total Volumes 2032

Appendix J – Left Turn Lane Assessment

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1 Introduction

1.1 Overview

Asurza Engineers Ltd. was retained by the proponent to undertake a traffic impact study for the proposed residential subdivision near Highway 62 in Belleville, Ontario. Since the development is located within the jurisdiction of the Ministry of Transportation, Ontario (MTO), the report was prepared for the review and approval by the City and the MTO to permit the proposed development.

1.2 Objectives

The purpose of this study is to determine any traffic impact the proposed development may generate on the adjacent roadways and adjacent intersections, as well as to identify the required improvements to maintain acceptable operational levels on the roadways within the study area.

The general scope of this study includes the following key elements:

- Establish baseline traffic conditions for the study area.
- Estimate the traffic growth for future planning horizons.
- Estimate the traffic generated by the proposed development.
- Estimate the total future traffic and identify impacts within the study area.
- Provide recommendations to address any deficiencies, if identified.

To achieve these objectives, the traffic study makes use of accepted methodologies and procedures including informational reports, publications from recognized institutions, recommended best practice manuals and MTO general guidelines for preparation of traffic impact studies.



2 Existing Conditions

2.1 Study Area

According to the 2021 census, Belleville has a population of approximately 55,071 permanent residents and is located approximately 170 km east of Toronto, Ontario.

The study area is located in the northern end of the city's urban boundaries, past Highway 401 and in proximity to Highway 62. See **Exhibit 1**.

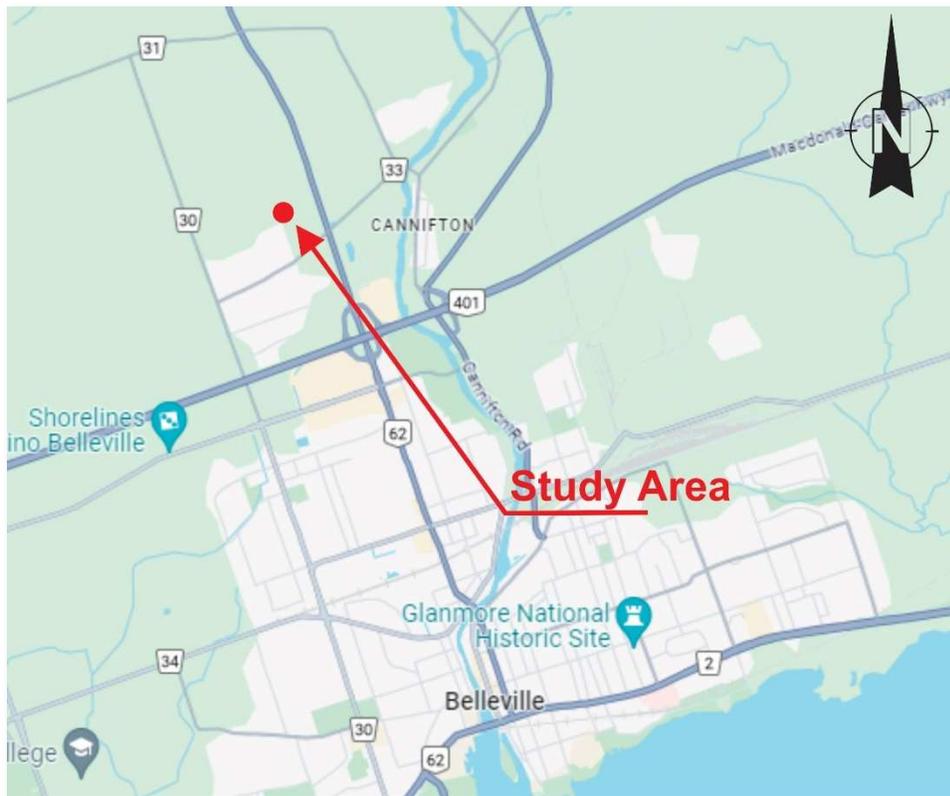


Exhibit 1: Study Area.

2.2 The Site

The subject site is located between an established commercial area and a recently built-out residential subdivision on the west. Both will serve as the entry points for vehicles arriving to/leaving from the site via Hwy 62 and Sidney St (Regional Rd 30). See **Exhibit 2**.



Exhibit 2: The Site.

The site is currently a vacant land of approximately 8 hectares. At its completion, the development is planned to include single family attached and detached houses, low-rise multifamily housing, and a low-rise residential building with ground-floor commercial space. See *Appendix A – Preliminary Concept Plan and Draft Plan of Subdivision* for further details.

2.3 Existing Roadway Network

The following roads are part of the study area:

Highway 62, known locally as North Front Street, is a five lane north-south arterial roadway (two lanes in each direction, and a central two-way left turn lane). Within the study area, Hwy 62 presents a rural cross section, utilizing granular shoulders, ditches, and culverts for water drainage. Hwy 62 provides driveway access to various commercial land uses near the subject site. From the north, Hwy 62 has a posted speed limit of 80 km/h, before transitioning to 70 km/h approx. 750 metres south of Farnham Rd, and to 60 km/h approx. 200 metres north of Maitland Dr.

Maitland Drive is a two lane (one lane in each direction) east-west roadway, providing access to residences and also serving an industrial park. This road intersects with Hwy 62 to form a 4-leg intersection controlled by traffic signals. Within the study area, Maitland Dr presents a rural cross section, and a posted speed of 50 km/h has been identified.

Farnham Road is a two lane (one lane in each direction) roadway. Within the study area, this road shows a rural cross section, intersecting with Hwy 62 to form a 4-leg intersection controlled by traffic signals. Posted speeds of 60 km/h and 50 km/h have been noted on west leg and the east leg of the intersection, respectively.

Roy Boulevard is a short dead-end road acting as a driveway for a few businesses and car dealerships. No posted speed was identified for Roy Blvd; therefore, 50 km/h is adopted in accordance with the Ontario Highway Traffic Act.

Sidney Street North is a two lane (one lane in each direction) roadway that provides vehicles with an alternative north/south travel route parallel to Hwy 62. Within the study area, Sidney St N presents a rural cross section, and a posted speed of 60 km/h has been identified. Kempton Ave intersects Sidney St N to form a “T”-type intersection with a stop sign on Kempton Ave.

Kempton Avenue is a two lane (one lane in each direction) short local road, providing residents with access to their driveways, as well as access to other roads within the subdivision. No posted speed was identified for Kempton Ave; therefore, 50 km/h is adopted.



2.4 Traffic Data

Traffic movement counts undertaken on May 10, 2022, were provided by MTO for the intersections of Hwy 62 at Farnham Rd, Roy Blvd, and Maitland Dr. A growth rate of 1.5% was used to project the counts to 2024. A video-based count was performed by Asurza Engineers on January 18, 2024, to gain data for Sidney St / Kempton Ave. See *Appendix B – Traffic Data*.

In addition, the traffic signal timings at Hwy 62 / Farnham Rd and Hwy 62 / Maitland Dr were provided by MTO. See *Appendix C – Existing Signal Timings*.

The existing lane configuration at the subject intersections are shown in ***Exhibit 3***, and the existing 2024 traffic volumes for the AM and PM peak hours are shown in ***Exhibits 4 and 5***.



Existing Lane Configuration

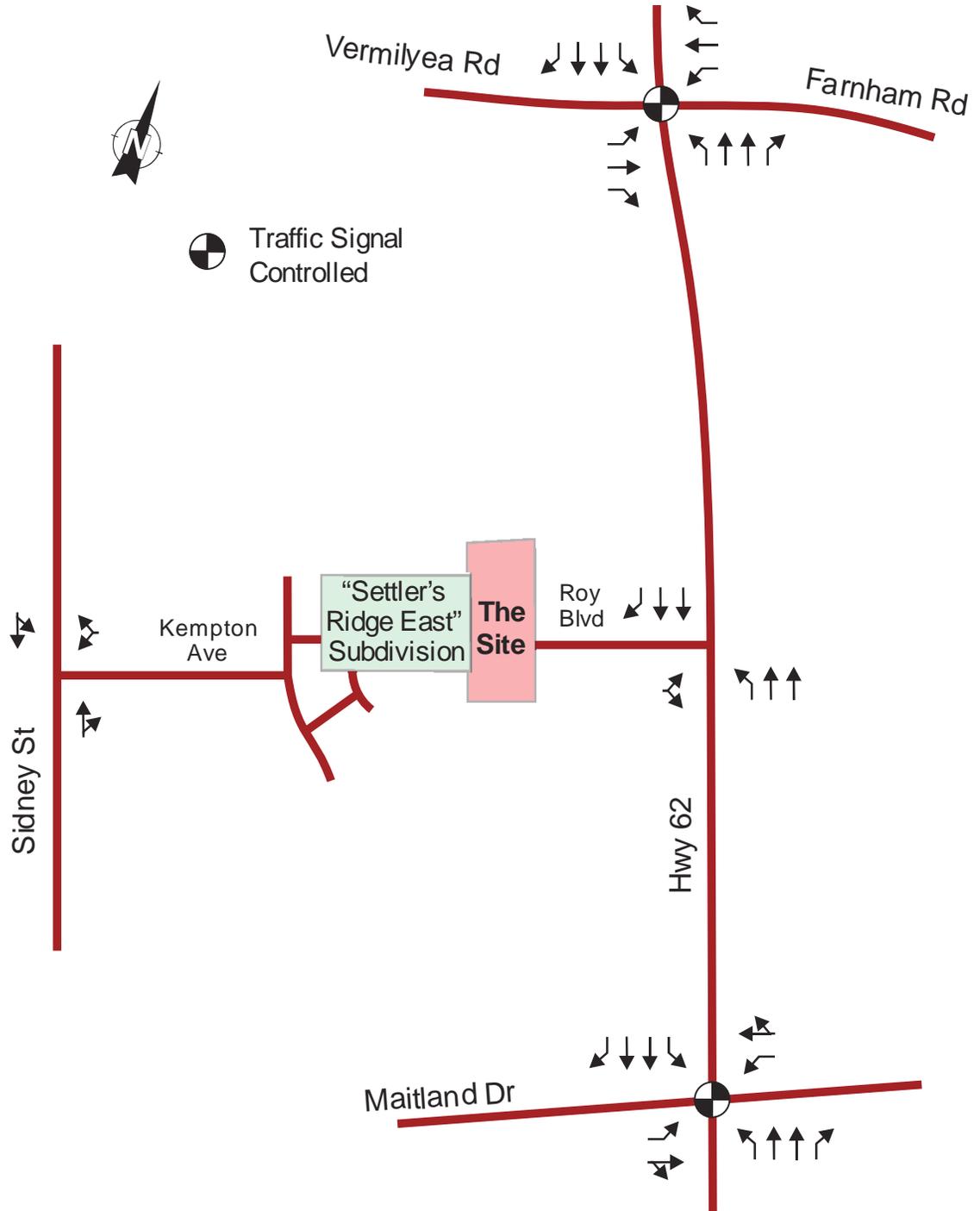


Exhibit 3: Existing Lane Configuration at Intersections.



Existing AM Peak Hour Volumes, 2024

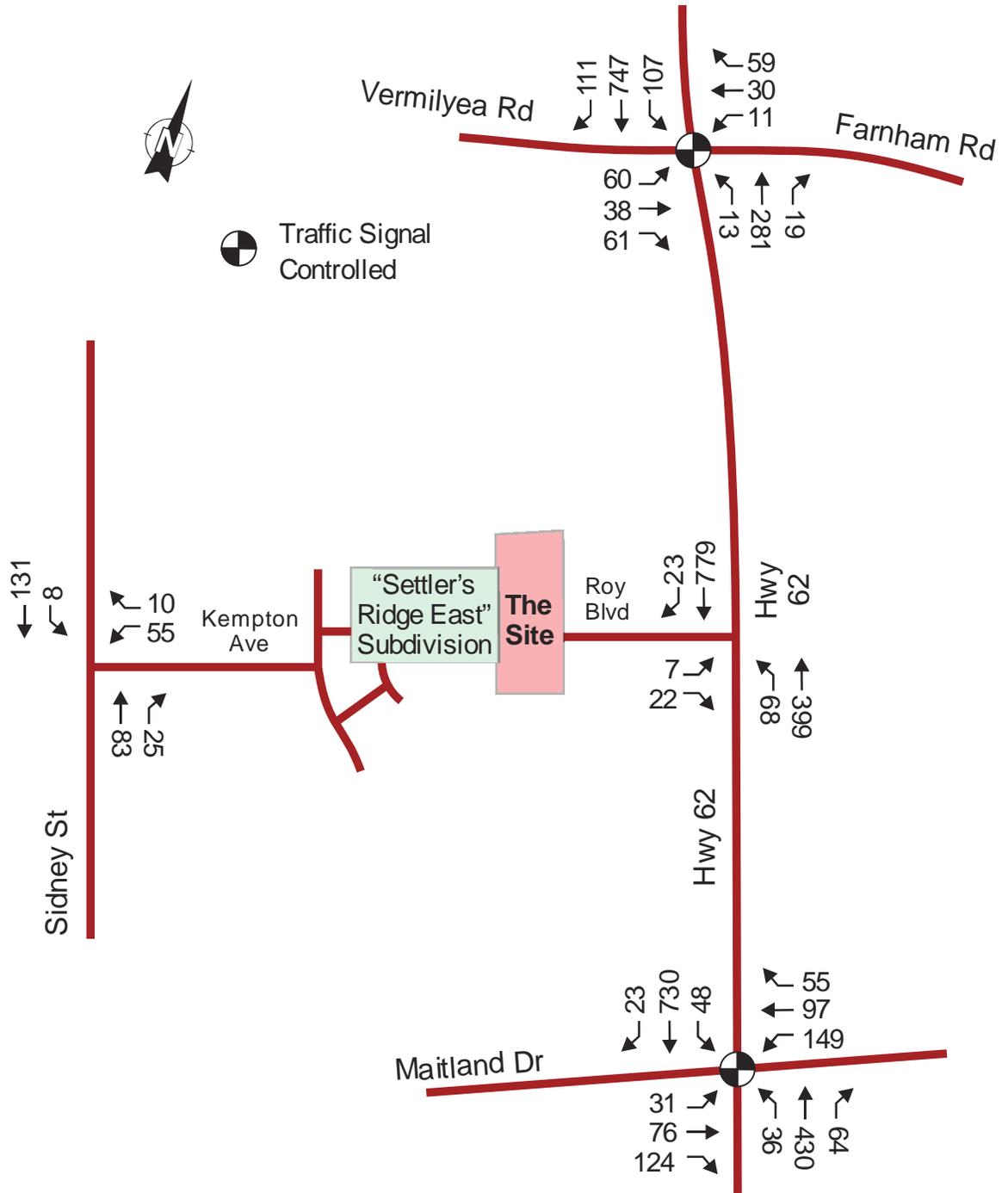


Exhibit 4: Existing AM Peak Hour Volumes, 2024.



Existing PM Peak Hour Volumes, 2024

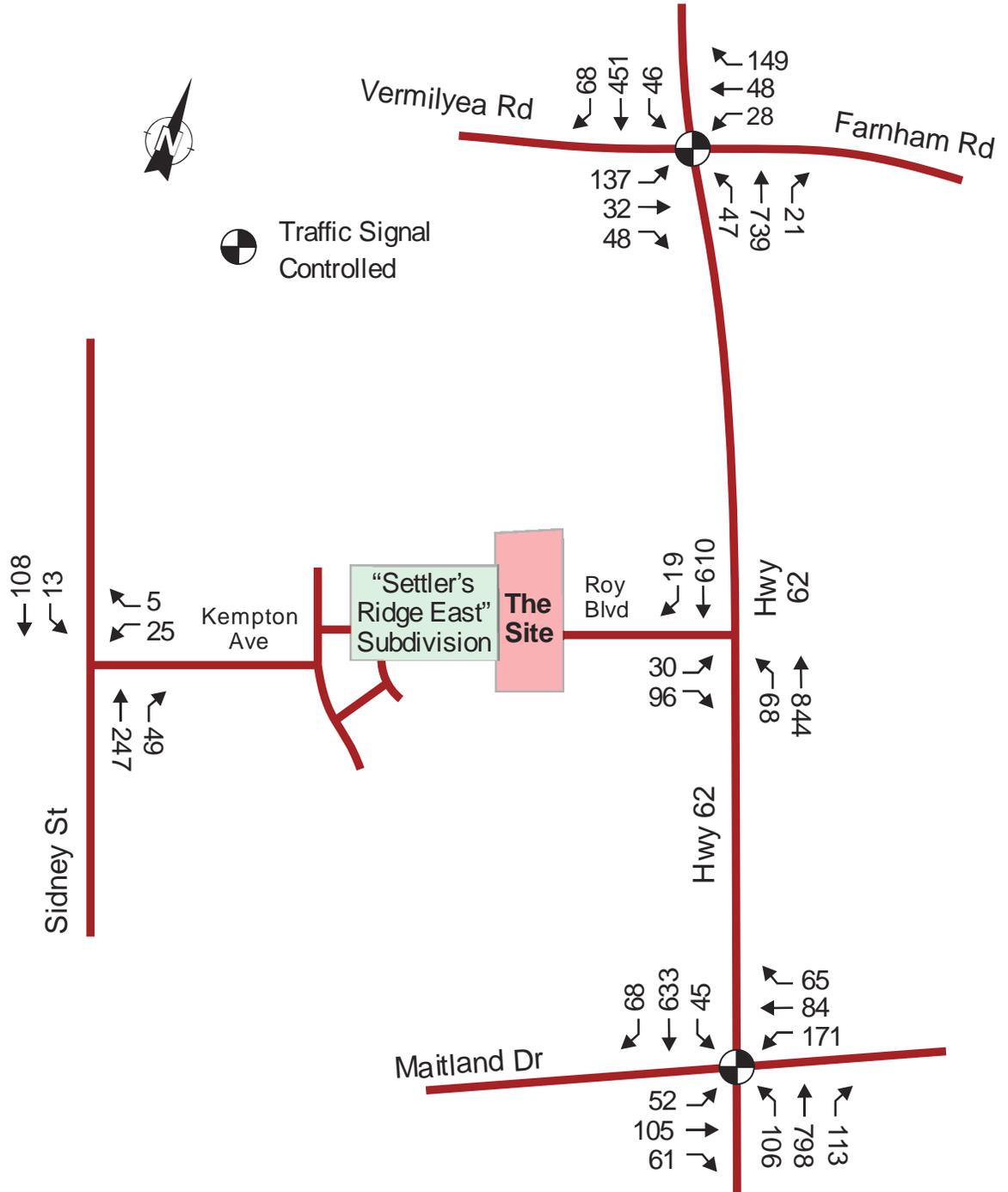


Exhibit 5: Existing PM Peak Hour Volumes, 2024.



2.5 Existing Traffic Operations

Intersection level of service (LOS) is a recognized method of qualifying the efficiency of traffic flow at intersections. The assigned LOS is determined on the delay caused by the control system experienced by individual vehicles executing the various movements. The delay is related to the number of vehicles desiring to make a particular movement, compared to the estimated capacity for that movement.

Table 1 shows the LOS criteria for intersections, ranging from ‘A’ to ‘F,’ where ‘A’ represents ideal traffic and ‘F’ represents extreme congestion.

LOS	Signalized Control Delay (sec/veh)	Unsignalized Control Delay (sec/veh)
A	0 - 10	0 - 10
B	> 10 - 20	> 10 - 15
C	> 20 - 35	> 15 - 25
D	> 35 - 55	> 25 - 35
E	> 55 - 80	> 35 - 50
F	> 80	> 50

Table 1: Level of Service Definition.

The intersection analysis, based on the Highway Capacity Manual (HCM), considers the following:

- The volume to capacity (v/c) ratio for the intersection and for each movement.
- The average delay in seconds for each movement and overall delay of the intersection.
- The 95th percentile queue length for each movement.
- The level of service for each movement and overall level of service of the intersection.

The existing operations for the subject intersections were evaluated in the Synchro 11 software using the existing traffic volumes for the AM and PM peak hours, and the results are summarized below in **Table 2**.



		Intersection Capacity, Existing Volumes, 2024							
		AM Peak Hour				PM Peak Hour			
		V/C	Delay (s)	Q ₉₅ (m)	LOS	V/C	Delay (s)	Q ₉₅ (m)	LOS
Highway 62 / Farnham Rd / Vermilyea Rd / (signalized)	EB-L	0.15	26.4	20.9	C	0.32	29.0	42.5	C
	EB-T	0.07	24.8	14.2	C	0.06	24.6	12.6	C
	EB-R	0.11	2.9	5.4	A	0.09	1.5	2.4	A
	WB-L	0.03	24.5	6.3	C	0.07	25.1	11.6	C
	WB-T	0.05	24.6	12.0	C	0.08	24.9	16.8	C
	WB-R	0.12	2.8	5.0	A	0.25	5.0	14.4	A
	NB-L	0.05	6.5	2.0	A	0.11	5.5	3.4	A
	NB-T	0.25	11.0	11.0	B	0.58	12.3	21.7	B
	NB-R	0.04	0.1	0.0	A	0.04	0.1	0.0	A
	SB-L	0.21	13.4	21.6	B	0.15	13.0	11.0	B
	SB-T	0.50	29.0	96.4	C	0.37	25.1	55.2	C
	SB-R	0.19	4.7	11.8	A	0.12	3.6	6.9	A
	Overall	0.60	20.0	-	B	0.58	16.0	-	B
Highway 62 / Roy Blvd (stop control)	EB-LR	0.07	13.5	1.8	B	0.25	13.8	7.9	B
	NB-L	0.10	10.2	2.6	B	0.08	9.3	2.1	B
	NB-T	0.13	0.0	0.0	A	0.27	0.0	0.0	A
	SB-T	0.33	0.0	0.0	A	0.26	0.0	0.0	A
	SB-R	0.18	0.0	0.0	A	0.14	0.0	0.0	A
Highway 62 / Maitland Dr (signalized)	EB-L	0.08	25.1	12.6	C	0.14	25.9	18.6	C
	EB-TR	0.35	18.3	42.4	B	0.29	23.2	42.3	C
	WB-L	0.45	32.9	49.3	C	0.47	33.0	55.2	C
	WB-TR	0.28	22.8	39.1	C	0.27	20.9	36.6	C
	NB-L	0.11	12.6	9.0	B	0.30	14.4	21.3	B
	NB-T	0.33	22.0	51.5	C	0.56	25.6	101.3	C
	NB-R	0.10	3.0	6.2	A	0.17	5.9	13.9	A
	SB-L	0.11	23.0	19.0	C	0.18	22.2	18.5	C
	SB-T	0.49	41.6	120.1	D	0.49	40.8	99.1	D
	SB-R	0.03	8.7	2.5	A	0.06	11.2	8.7	B
Overall	0.49	29.3	-	C	0.56	28.1	-	C	
Sidney St / Kempton Ave (stop control)	WB-LR	0.10	10.4	2.6	B	0.06	11.5	1.4	B
	NB-TR	0.07	0.0	0.0	A	0.19	0.0	0.0	A
	SB-TL	0.01	0.5	0.2	A	0.01	0.9	0.3	A

Table 2: Intersection Capacity, Existing Volumes, 2024.

Results show that movements at intersections are currently operating well, with acceptable control delays during peak hours. All movements show LOS “C” or better, except the southbound thru at the Hwy 62/Maitland Dr which shows LOS “D”. All resulting v/c ratios are well below 0.85.



3 Background Traffic Volumes and Operations

3.1 Background Traffic Volumes

In order to establish base conditions for the comparison and evaluation of future scenarios, it is necessary to review the results of traffic operations over time. To maintain a conservative estimate of future traffic, a growth rate of 1.5% per year will be implemented for the purposes of this study.

The rate used is based on historical AADT data for Highway 62 found in the MTO publication titled, “Provincial Highways Traffic Volumes 1988-2016.” The background traffic volumes were estimated by applying the growth rate to the existing traffic volumes. See *Appendix D – Traffic Volume Projections*.

For this study, the horizon years for analysis are 2027 and 2032, which are expected to coincide with the full build-out of the proposed development, and five years after the full build-out, respectively.

Also, to maintain accuracy in the analysis, the ongoing build-out of the nearby “Settler’s Ridge East Phase 2” subdivision should be accounted for. As of the date of this report, the subdivision has finished its Phase 1 build-out; therefore, the new trips resulting from Phase 2 (36 single detached units and 31 townhome units) have been estimated and distributed to the Sidney St / Kempton Ave intersection.

The following **Exhibits 6 to 9** show the projected traffic volumes for the morning and afternoon peak hours for the horizon years 2027 and 2032.



Background AM Peak Hour Volumes, 2027

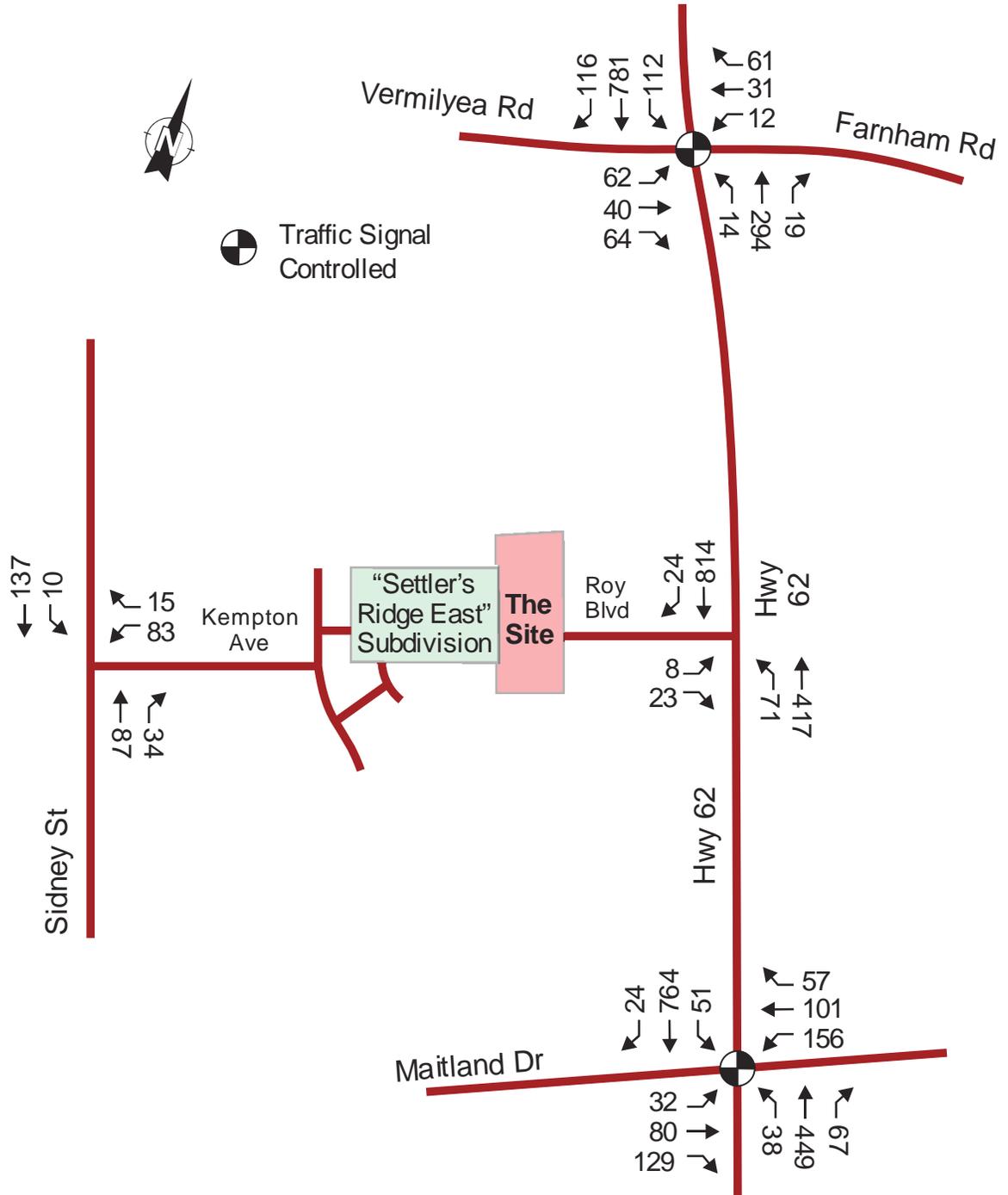


Exhibit 6: Background AM Peak Hour Volumes, 2027.



Background PM Peak Hour Volumes, 2027

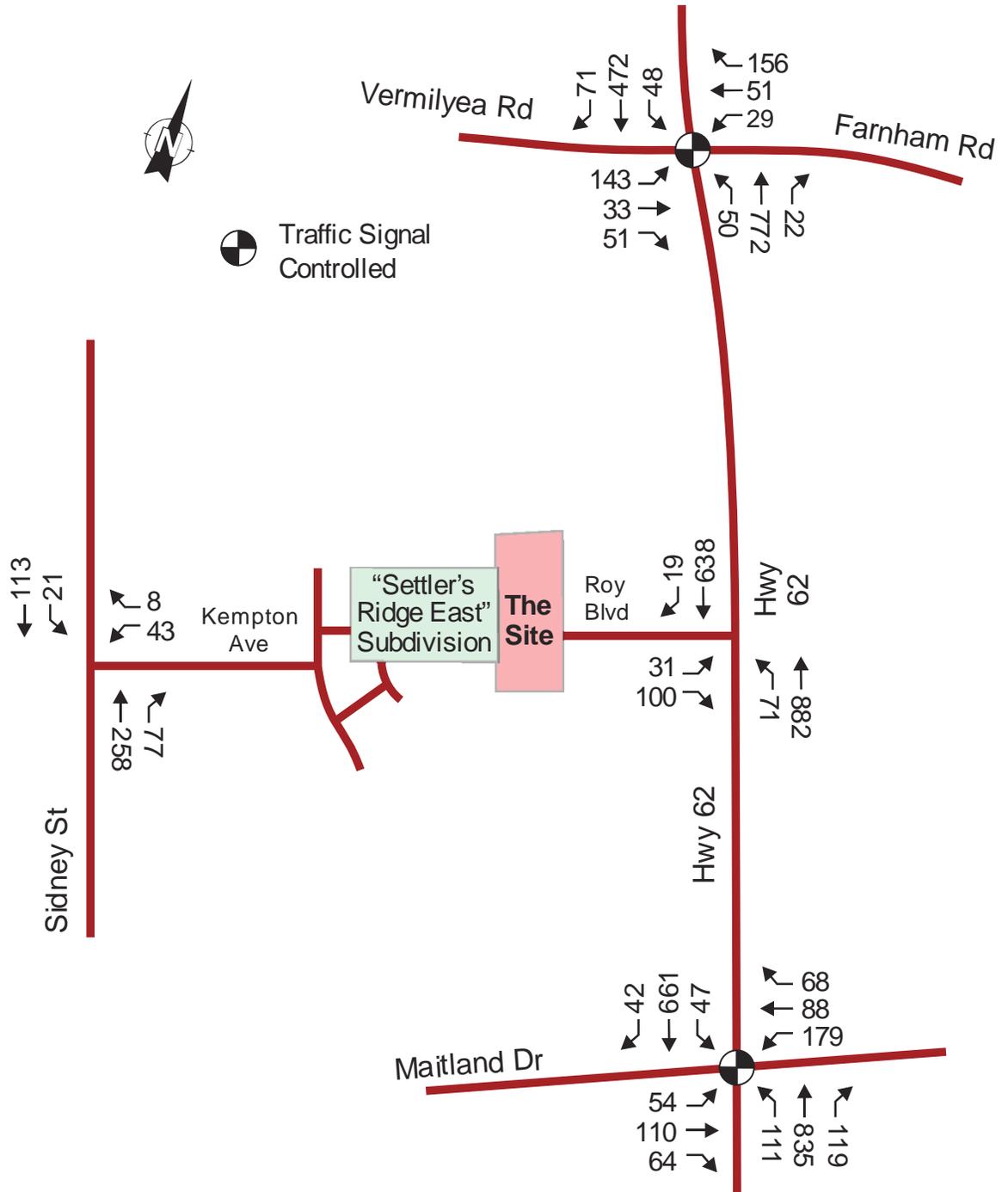


Exhibit 7: Background PM Peak Hour Volumes, 2027.



Background AM Peak Hour Volumes, 2032

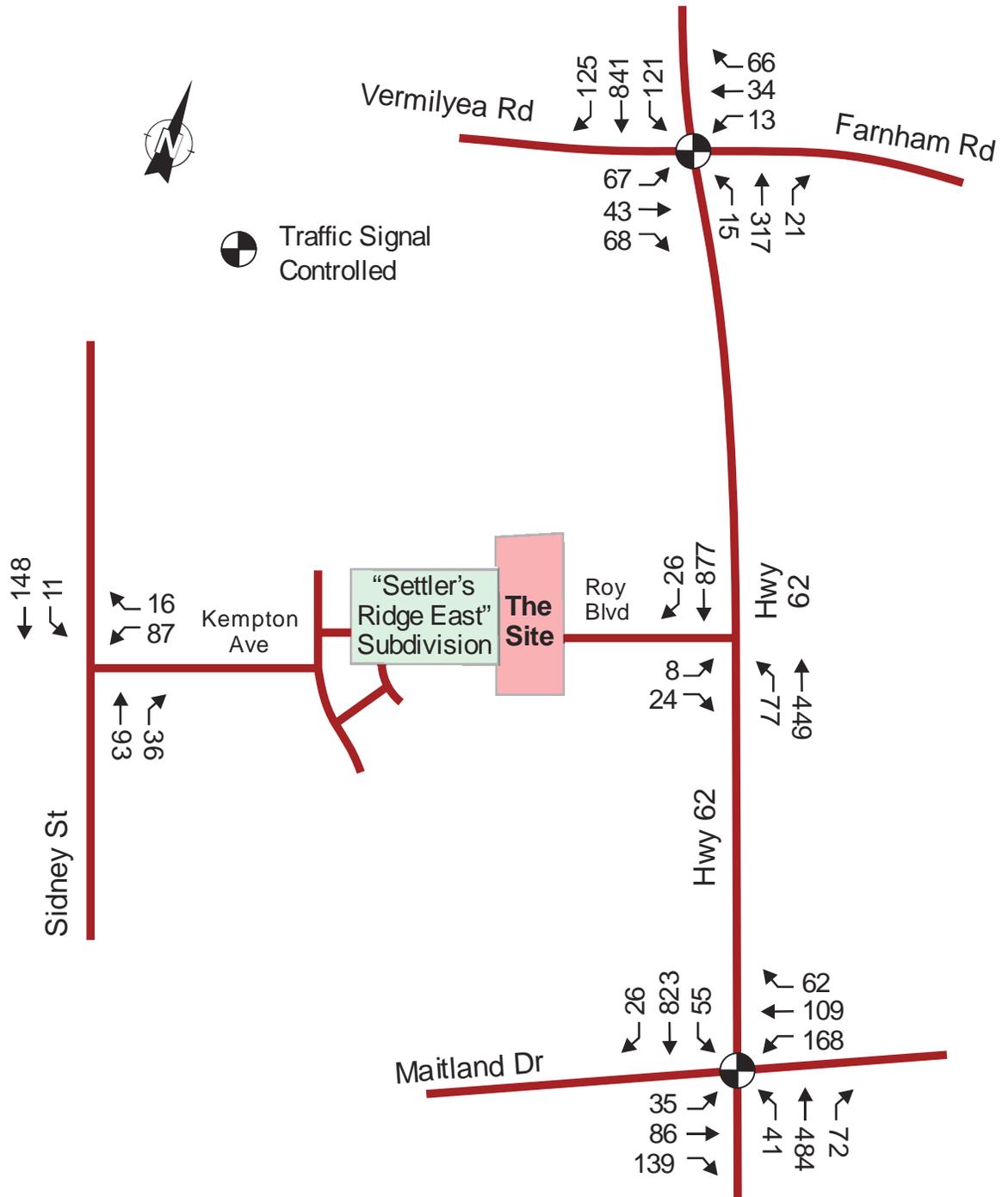


Exhibit 8: Background AM Peak Hour Volumes, 2032.



Background PM Peak Hour Volumes, 2032

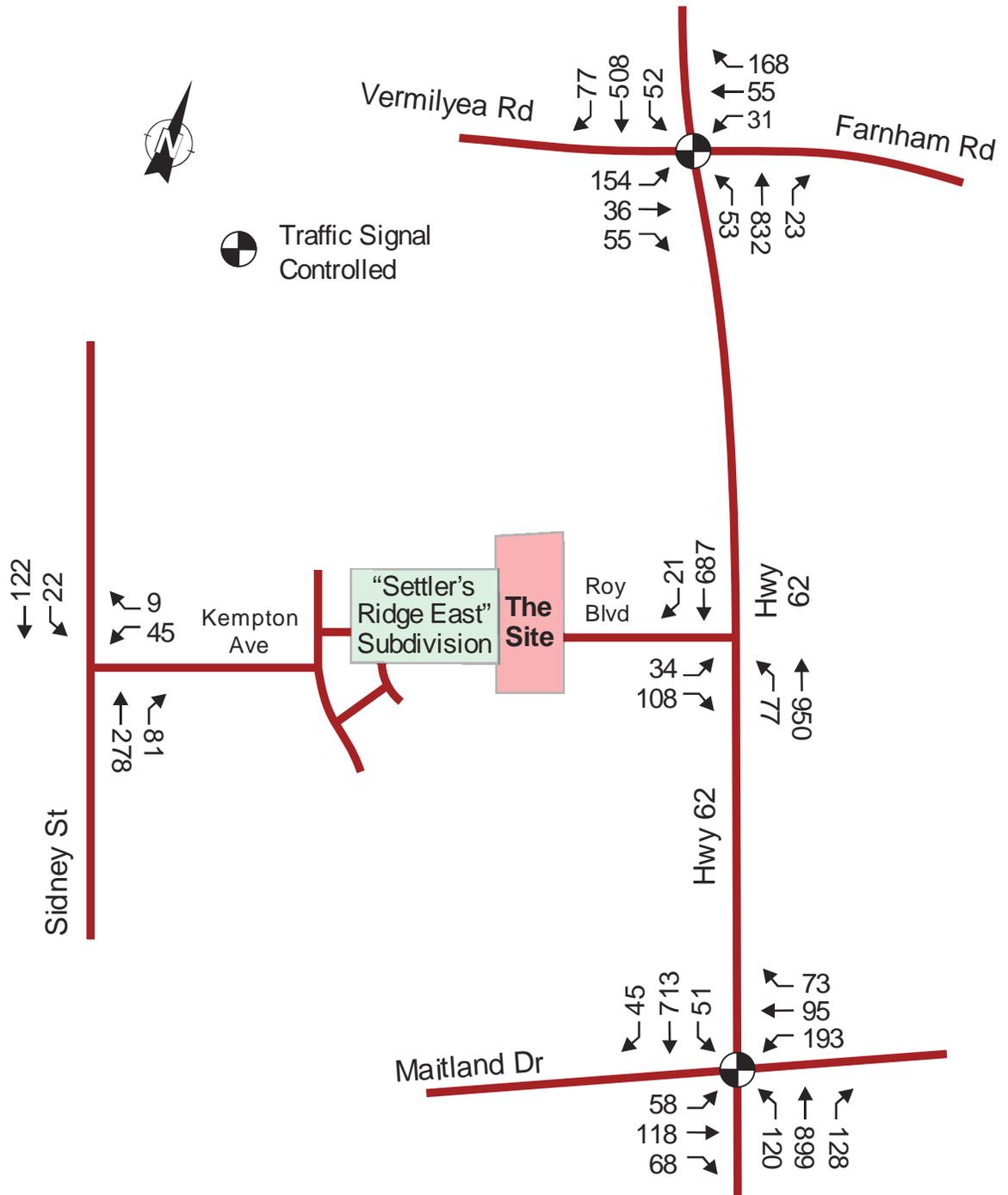


Exhibit 9: Background PM Peak Hour Volumes, 2032.



3.2 Background Traffic Operations

The summary of results for the background traffic operations in 2027 and 2032 are shown in the following *Tables 3 and 4*.

		Intersection Capacity, Background Volumes, 2027							
		AM Peak Hour				PM Peak Hour			
		V/C	Delay (s)	Q ₉₅ (m)	LOS	V/C	Delay (s)	Q ₉₅ (m)	LOS
Highway 62 / Farnham Rd / Vermilyea Rd / (signalized)	EB-L	0.16	26.4	21.4	C	0.33	29.3	44.0	C
	EB-T	0.07	24.9	14.8	C	0.06	24.6	12.8	C
	EB-R	0.12	3.4	6.4	A	0.09	1.8	3.2	A
	WB-L	0.03	24.5	6.4	C	0.08	25.1	12.1	C
	WB-T	0.06	25.6	12.3	C	0.08	25.0	17.7	C
	WB-R	0.12	2.9	5.4	A	0.26	5.0	14.7	A
	NB-L	0.05	6.6	1.9	A	0.12	5.5	3.4	A
	NB-T	0.26	11.0	11.5	B	0.61	12.2	22.5	B
	NB-R	0.04	0.1	0.0	A	0.04	0.1	0.0	A
	SB-L	0.22	13.5	22.6	B	0.17	13.2	11.3	B
	SB-T	0.62	29.7	101.8	C	0.39	25.3	57.9	C
	SB-R	0.19	4.6	12.0	A	0.13	3.9	7.5	A
	Overall	0.62	20.3	-	C	0.61	16.0	-	B
Highway 62 / Roy Blvd (stop control)	EB-LR	0.08	14.0	2.0	B	0.27	14.3	8.7	B
	NB-L	0.10	10.5	2.8	B	0.09	9.5	2.3	B
	NB-T	0.13	0.0	0.0	A	0.28	0.0	0.0	A
	SB-T	0.35	0.0	0.0	A	0.27	0.0	0.0	A
	SB-R	0.19	0.0	0.0	A	0.15	0.0	0.0	A
Highway 62 / Maitland Dr (signalized)	EB-L	0.09	25.2	12.9	C	0.14	26.1	19.2	C
	EB-TR	0.36	19.0	44.9	B	0.30	23.7	44.9	C
	WB-L	0.48	34.0	52.3	C	0.51	34.1	58.3	C
	WB-TR	0.29	23.3	41.1	C	0.28	21.3	38.6	C
	NB-L	0.12	12.7	9.4	B	0.33	14.8	22.4	B
	NB-T	0.35	22.3	54.3	C	0.59	26.2	107.4	C
	NB-R	0.11	3.4	6.7	A	0.17	6.4	15.1	A
	SB-L	0.12	22.7	19.5	C	0.20	22.4	18.9	C
	SB-T	0.52	41.9	125.0	D	0.51	41.3	103.2	D
	SB-R	0.03	8.6	2.4	A	0.07	11.9	9.3	B
Overall	0.52	29.7	-	C	0.59	28.6	-	C	
Sidney St / Kempton Ave (stop control)	WB-LR	0.14	10.7	4.0	B	0.10	12.2	2.7	B
	NB-TR	0.08	0.0	0.0	A	0.21	0.0	0.0	A
	SB-TL	0.01	0.6	0.2	A	0.02	1.4	0.5	A

Table 3: Intersection Capacity, Background Volumes, 2027.

		Intersection Capacity, Background Volumes, 2032							
		AM Peak Hour				PM Peak Hour			
		V/C	Delay (s)	Q ₉₅ (m)	LOS	V/C	Delay (s)	Q ₉₅ (m)	LOS
Highway 62 / Farnham Rd / Vermilyea Rd / (signalized)	EB-L	0.17	26.7	22.9	C	0.36	29.8	47.4	C
	EB-T	0.08	25.0	15.8	C	0.06	24.7	13.6	C
	EB-R	0.13	4.0	7.3	A	0.10	2.4	4.2	A
	WB-L	0.04	24.5	6.8	C	0.08	25.2	12.6	C
	WB-T	0.06	24.7	13.1	C	0.09	25.1	18.8	C
	WB-R	0.13	3.7	6.8	A	0.28	4.9	15.1	A
	NB-L	0.06	6.5	2.0	A	0.13	5.5	3.4	A
	NB-T	0.28	11.1	12.2	B	0.66	12.7	24.0	B
	NB-R	0.04	0.1	0.0	A	0.04	0.1	0.0	A
	SB-L	0.25	13.8	24.3	B	0.20	13.6	12.1	B
	SB-T	0.67	30.9	111.8	C	0.42	25.8	62.7	C
	SB-R	0.21	5.3	13.4	A	0.14	4.6	8.9	A
	Overall	0.67	21.0	-	C	0.66	16.5	-	B
Highway 62 / Roy Blvd (stop control)	EB-LR	0.09	14.7	2.2	B	0.31	15.4	10.4	C
	NB-L	0.12	10.9	3.3	B	0.10	9.8	2.7	A
	NB-T	0.14	0.0	0.0	A	0.30	0.0	0.0	A
	SB-T	0.37	0.0	0.0	A	0.29	0.0	0.0	A
	SB-R	0.20	0.0	0.0	A	0.16	0.0	0.0	A
Highway 62 / Maitland Dr (signalized)	EB-L	0.10	25.4	13.6	C	0.16	26.3	20.4	C
	EB-TR	0.39	19.9	49.2	B	0.32	24.2	47.9	C
	WB-L	0.54	36.4	57.3	D	0.56	36.1	63.8	D
	WB-TR	0.31	24.0	44.4	C	0.30	22.2	41.5	C
	NB-L	0.15	13.0	10.0	B	0.38	15.7	23.7	B
	NB-T	0.37	22.8	59.2	C	0.63	27.3	118.4	C
	NB-R	0.11	4.0	7.8	A	0.19	7.1	17.2	A
	SB-L	0.14	22.2	20.1	C	0.23	23.0	20.0	C
	SB-T	0.59	44.7	134.1	D	0.55	42.2	111.2	D
	SB-R	0.04	9.0	2.2	A	0.07	12.2	10.4	B
Overall	0.59	31.3	-	C	0.63	29.6	-	C	
Sidney St / Kempton Ave (stop control)	WB-LR	0.16	11.0	4.4	B	0.11	12.6	3.0	B
	NB-TR	0.08	0.0	0.0	A	0.23	0.0	0.0	A
	SB-TL	0.01	0.6	0.2	A	0.02	1.4	0.5	A

Table 4: Intersection Capacity, Background Volumes, 2032.

The results show that, without the proposed development, delays will moderately increase over time as expected. Throughout the study years, all movements will continue to operate with acceptable levels of service.



4 Proposed Development Traffic Forecasting

4.1 Traffic Impact Study Methodology

The traffic impact analysis was completed in accordance with the methodologies published by the Transportation Research Board (TRB) and the Transportation Impact Analysis for Site Developments published by the Institute of Transportation Engineers (ITE).

Additionally, the MTO provides general guidelines for preparation of traffic impact studies; these guidelines were also consulted in order to comply with the requirements for this study.

The estimation of trips generated by the proposed development were derived from the Trip Generation Manual, 11th Edition, published by ITE.

4.2 Site Trip Generation

The land uses which most closely describe the proposed development are the following:

- Single-Family Detached Housing – ITE Code 210
- Single-Family Attached Housing – ITE Code 215
- Multifamily Housing (Low Rise) – ITE Code 220
- Low-Rise Residential w/ Ground Floor Commercial – ITE Code 230

The ITE trip generation rates and directional distributions for the above land uses are shown in **Table 5**. As per the ITE methodology, the fitted equations are used in place of the average rates where they are applicable.



TRIP GENERATION RATES BY LAND USE								
ITE Code	ITE Land Use	Unit of Measure	AM Peak Hr. of Adj. Street			PM Peak Hr. of Adj. Street		
			Rate	In	Out	Rate	In	Out
210	Single-Family Detached Housing	Dwelling Units	$\text{Ln}(T) = 0.91x + 0.12$	25%	75%	$\text{Ln}(T) = 0.94x + 0.27$	63%	37%
215	Single-Family Attached Housing	Dwelling Units	$T = 0.52x - 5.70$	25%	75%	$T = 0.60x - 3.93$	59%	41%
220	Multifamily Housing (Low Rise)	Dwelling Units	$T = 0.31x + 22.9$	24%	76%	$T = 0.43x + 20.6$	63%	37%
230	Low-Rise Residential w/ Ground-Floor Commercial	Dwelling Units	0.44	23%	77%	0.36	71%	29%

Table 5: ITE Trip Rates and Dir. Distributions per Land Use.

The estimated trips generated by the proposed development are shown in **Table 6**.

ESTIMATED NUMBER OF TRIPS BY LAND USE								
ITE Code	ITE Land Use	Total Units	AM Peak Hr. of Adj. Street			PM Peak Hr. of Adj. Street		
			Trips	In	Out	Trips	In	Out
210	Single-Family Detached Housing	50	40	10	30	52	33	19
215	Single-Family Attached Housing	26	8	2	6	12	7	5
220	Multifamily Housing (Low Rise)	106	55	13	42	66	42	24
230	Low-Rise Residential w/ Ground-Floor Commercial	20	9	2	7	7	5	2
NEW TRIPS GENERATED			112	27	85	137	87	50

Table 6: Estimation of Trips Generated by the Proposed Development.

According to the Trip Generation Manual, the descriptions for each land use are as follows:

A Single-Family Detached Housing site includes any single-family detached home on an individual lot.

A Single-Family Attached Housing site includes any single-family housing unit that shares a wall with an adjoining dwelling unit.

Low-Rise Multifamily Housing includes apartments, townhouses, and condominiums located within the same building with at least three other dwelling units and that have two or three floors (levels).



Low-Rise Residential with Ground-Floor Commercial includes any mixed-use multifamily housing building with two or three floors of residential living space and commercial space open to the public on the ground level.

The proposed development is estimated to generate an additional 112 and 137 new trips during the AM and PM peak hours, respectively.

4.3 Trip Distribution/Assignment

The vast majority of new trips generated are expected to be entering/exiting the development using Hwy 62 / Roy Blvd. For the purposes of this analysis, 95% of the new trips are assumed to enter/exit the development at that intersection, while the remaining 5% is assigned to Sidney St / Kempton Ave intersection.

From here, the new trips are distributed proportional to the existing travel patterns, for which the proportions of trips are shown in **Exhibits 10 and 11**.

Exhibits 12 and 13 show the new trips generated by the proposed development for the morning and afternoon peak hours, distributed according to the noted proportions.



Trip Distribution - AM Peak Hour

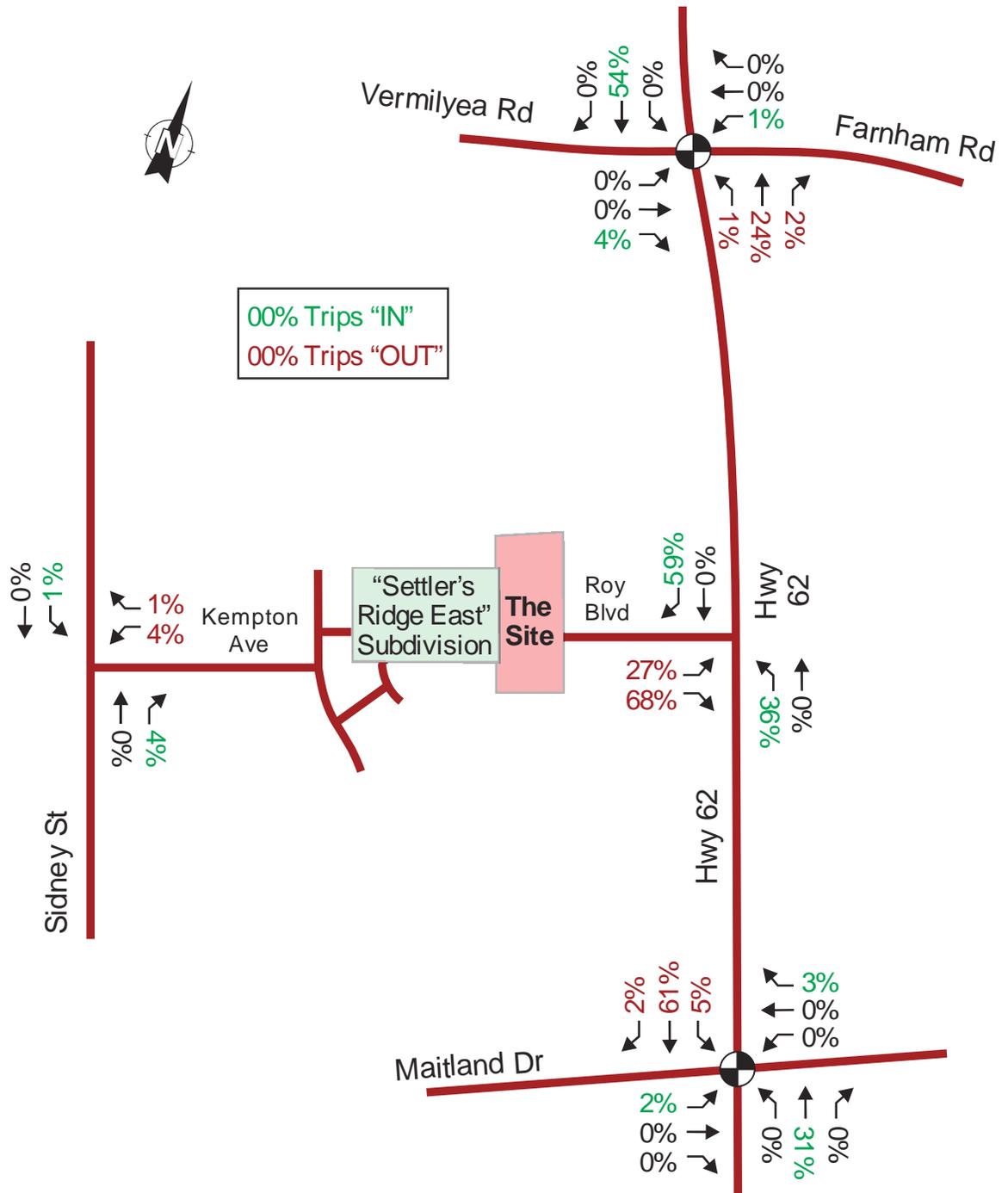


Exhibit 10: Trip Distribution, AM Peak Hour.



Trip Distribution - PM Peak Hour

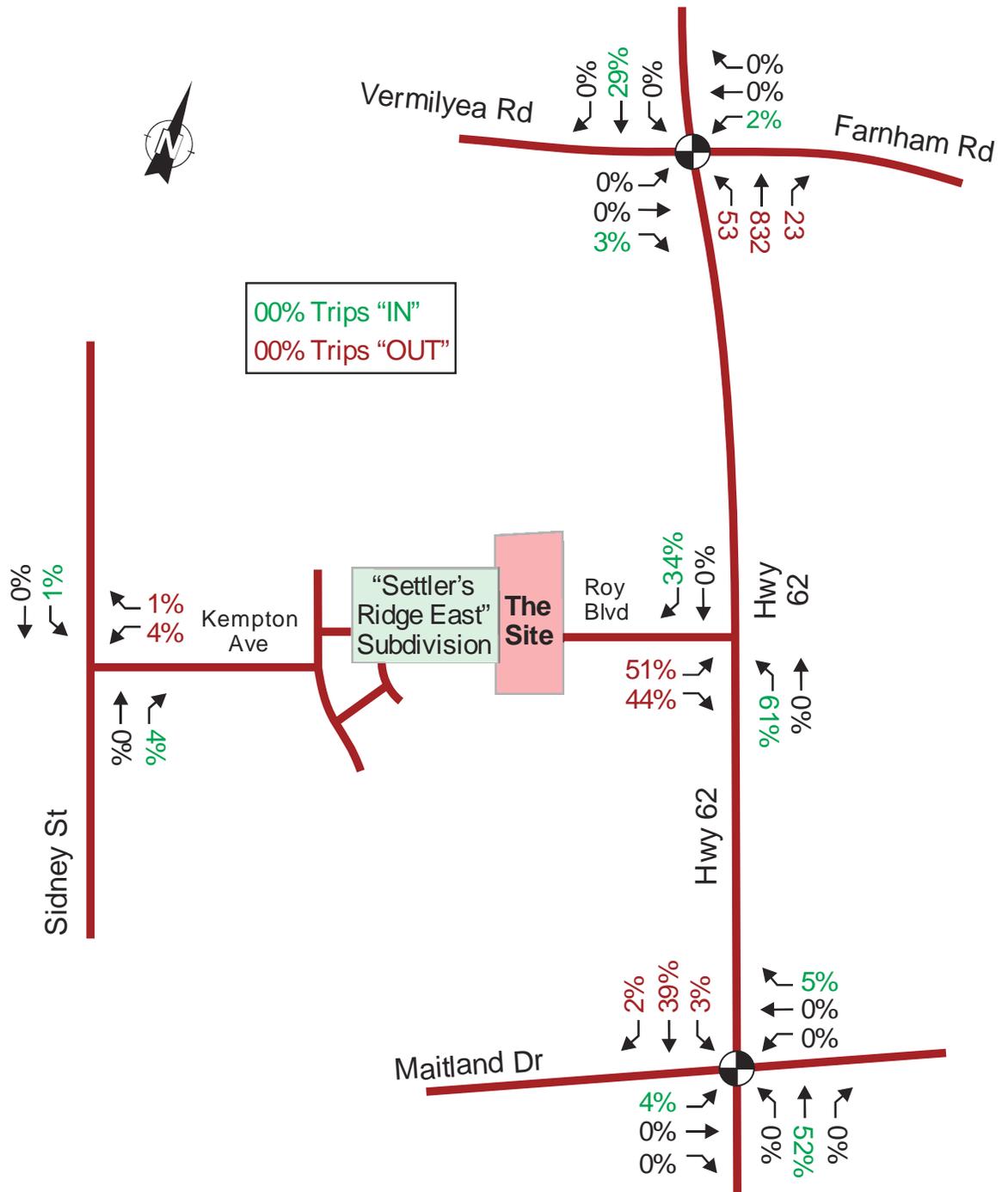


Exhibit 11: Trip Distribution, PM Peak Hour.



New Trips Generated, AM Peak Hour, 2027

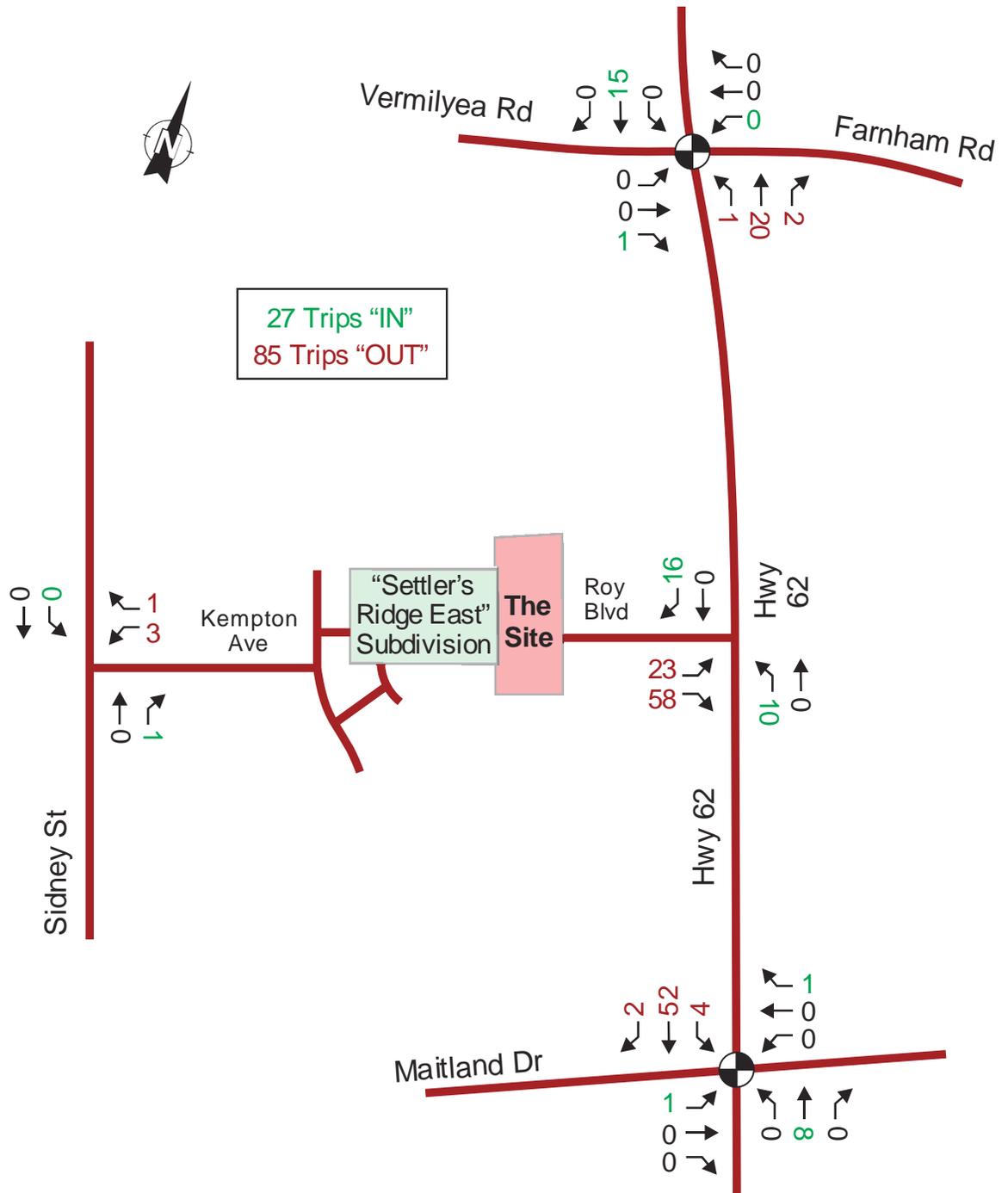


Exhibit 12: New Trips Generated, AM Peak Hour, 2027.



New Trips Generated, PM Peak Hour, 2027

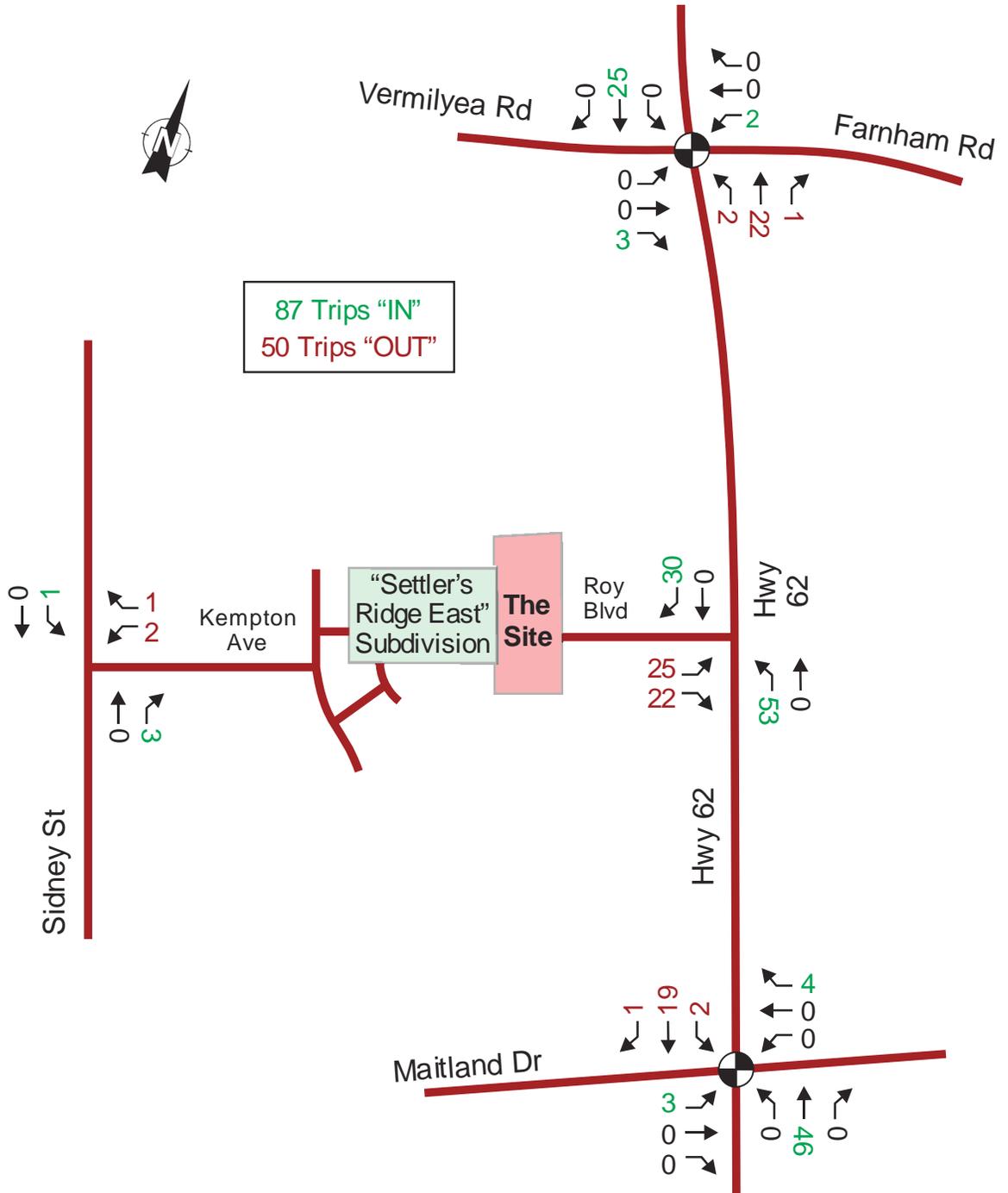


Exhibit 13: New Trips Generated, PM Peak Hour, 2027.



5 Future Traffic Volumes and Operations

5.1 Future Total Traffic Volumes

The future total traffic volumes are obtained by adding the background traffic volumes to the total new traffic volumes generated by the proposed development.

The total traffic volumes for the horizon years 2027 and 2032 are shown in ***Exhibits 14 to 17.***



Total AM Peak Hour Volumes, 2027

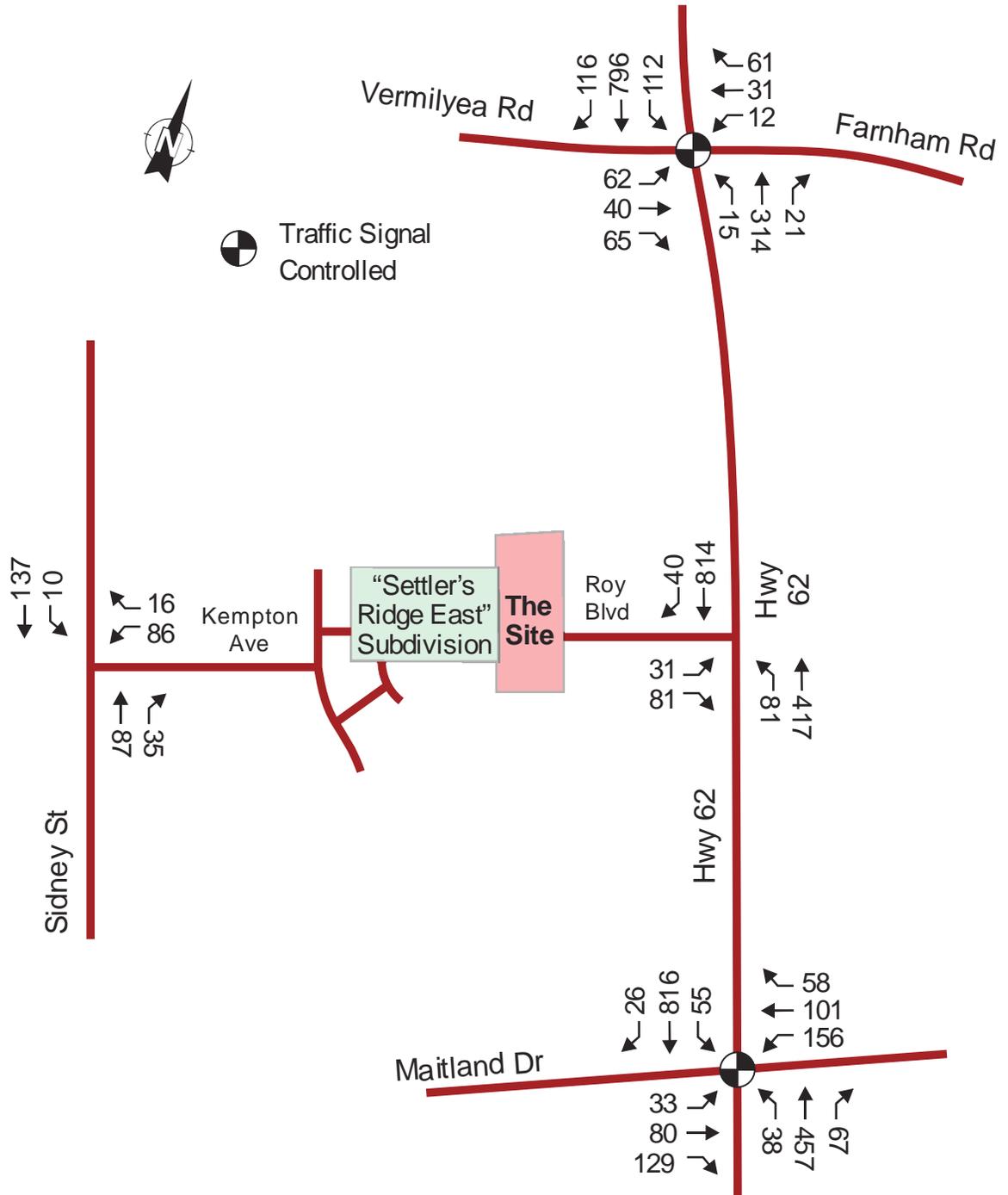


Exhibit 14: Total AM Peak Hour Volumes, 2027.



Total PM Peak Hour Volumes, 2027

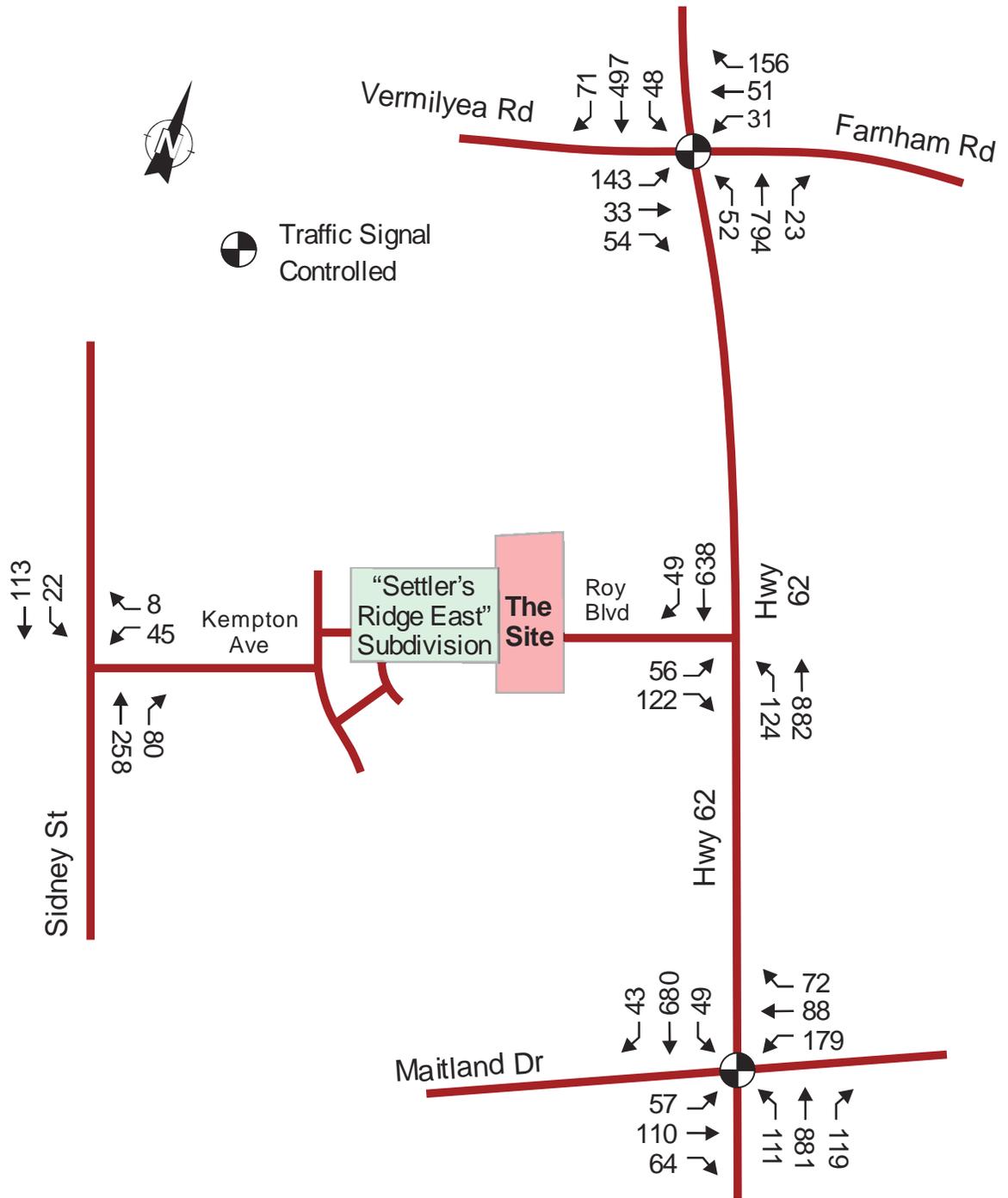


Exhibit 15: Total PM Peak Hour Volumes, 2027.

Total AM Peak Hour Volumes, 2032

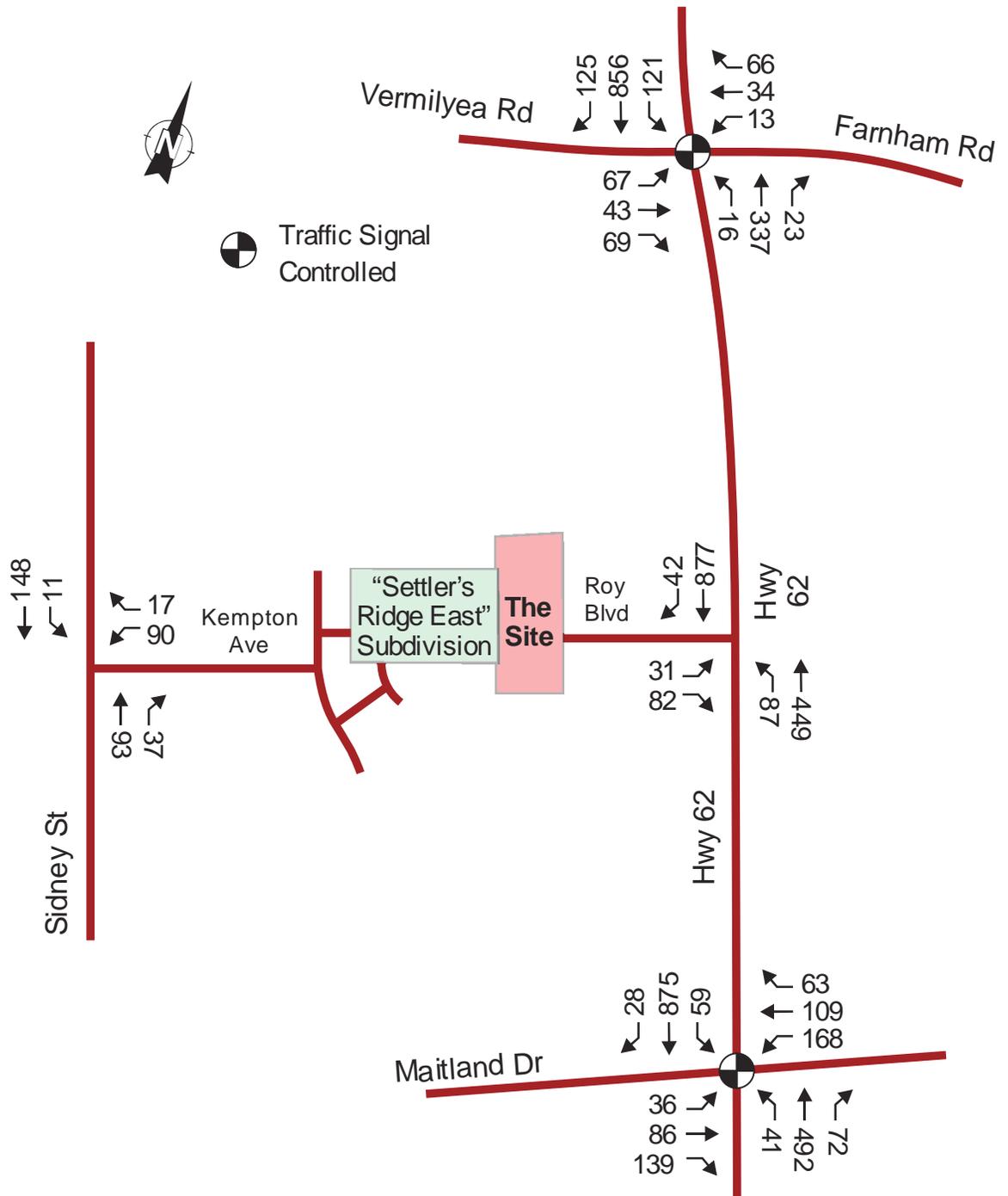


Exhibit 16: Total AM Peak Hour Volumes, 2032.



Total PM Peak Hour Volumes, 2032

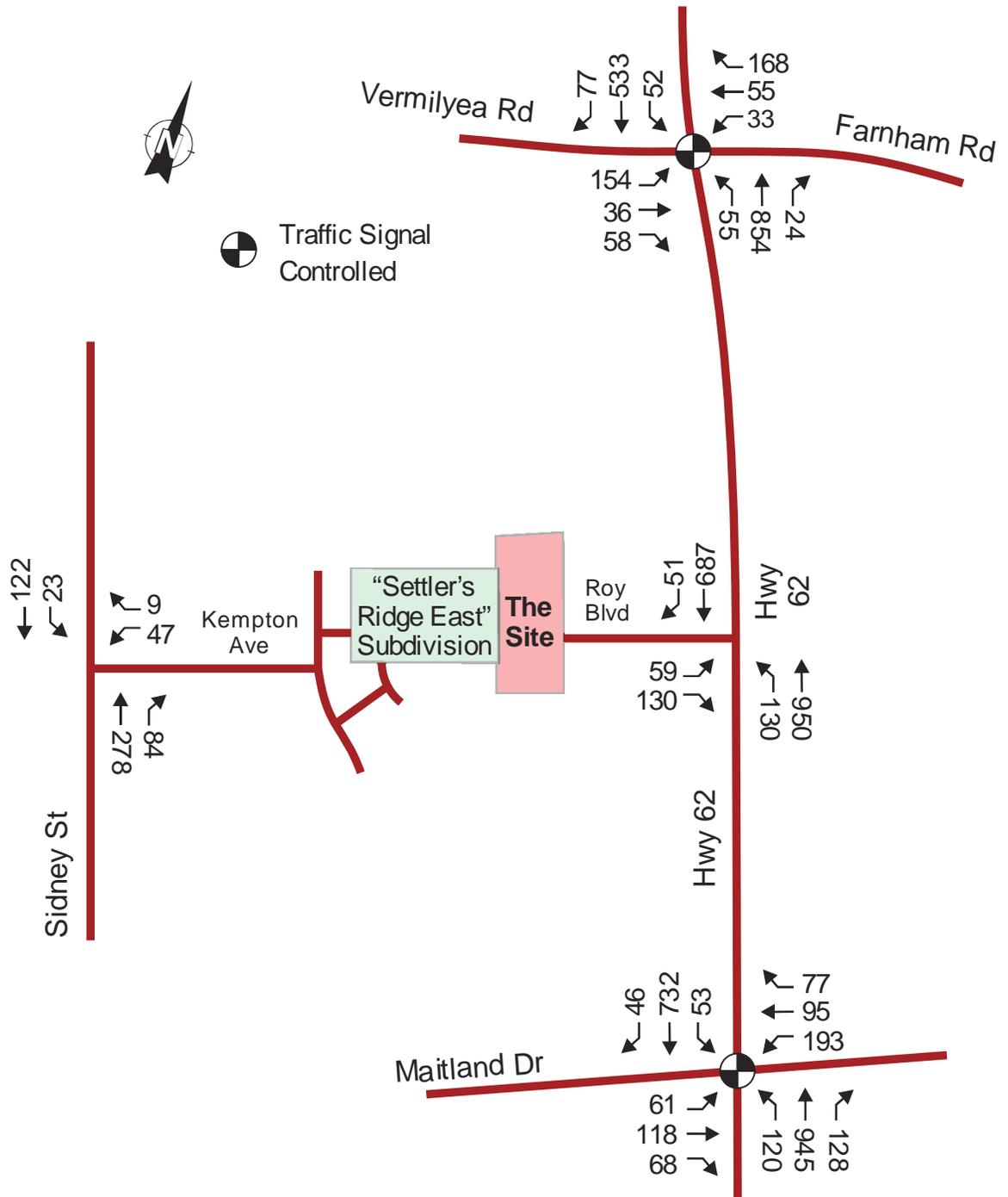


Exhibit 17: Total AM Peak Hour Volumes, 2032.



5.2 Future Total Traffic Operations

“General Guidelines for the Preparation of Traffic Impact Studies – Feb 2021,” an MTO publication, indicates that at signalized intersections, movements with a v/c ratio greater than 0.85 are deemed to be “critical” in terms of operations and shall be evaluated for possible improvements.

Tables 7 and 8 show a summary of the capacity analysis results including the total volumes (background traffic + new trips) for the horizon years 2027 and 2032.

The results show that, including the proposed development, the delays and LOS for all movements remain basically the same as the background scenarios. The most critical movements are expected to be the westbound-left and southbound-thru movements at Highway 62 / Maitland Dr, both showing LOS “D” by 2032 regardless of the proposed development.

Also, the v/c ratio for all movements will remain well below 0.85 throughout the study years.



		Intersection Capacity, Total Volumes, 2027							
		AM Peak Hour				PM Peak Hour			
		V/C	Delay (s)	Q ₉₅ (m)	LOS	V/C	Delay (s)	Q ₉₅ (m)	LOS
Highway 62 / Farnham Rd / Vermilyea Rd / (signalized)	EB-L	0.16	26.4	21.4	C	0.33	29.3	44.0	C
	EB-T	0.07	24.9	14.8	C	0.06	24.6	12.8	C
	EB-R	0.12	3.5	6.5	A	0.10	2.2	3.9	A
	WB-L	0.03	24.5	6.4	C	0.08	25.2	12.6	C
	WB-T	0.06	24.6	12.3	C	0.08	25.0	17.7	C
	WB-R	0.12	2.9	5.4	A	0.26	5.0	14.7	A
	NB-L	0.06	6.8	2.2	A	0.13	5.7	3.6	A
	NB-T	0.28	11.7	13.3	B	0.63	12.7	24.5	B
	NB-R	0.04	0.1	0.0	A	0.04	0.1	0.0	A
	SB-L	0.23	13.6	22.3	B	0.17	13.2	11.3	B
	SB-T	0.64	29.9	104.3	C	0.41	25.6	61.2	C
	SB-R	0.19	4.6	12.0	A	0.13	3.9	7.5	A
	Overall	0.64	20.5	-	C	0.63	16.4	-	B
Highway 62 / Roy Blvd (stop control)	EB-LR	0.29	16.9	9.4	C	0.42	18.3	16.4	C
	NB-L	0.12	10.6	3.3	B	0.16	10.0	4.5	A
	NB-T	0.13	0.0	0.0	A	0.28	0.0	0.0	A
	SB-T	0.35	0.0	0.0	A	0.27	0.0	0.0	A
	SB-R	0.20	0.0	0.0	A	0.17	0.0	0.0	A
Highway 62 / Maitland Dr (signalized)	EB-L	0.09	25.2	13.1	C	0.15	26.2	19.9	C
	EB-TR	0.36	19.0	44.9	D	0.30	23.7	44.9	C
	WB-L	0.48	34.0	52.3	C	0.51	34.1	58.3	C
	WB-TR	0.29	23.2	41.1	C	0.29	21.3	39.0	C
	NB-L	0.13	12.8	9.4	B	0.34	15.0	22.4	B
	NB-T	0.35	22.5	55.6	C	0.62	27.0	115.2	C
	NB-R	0.11	3.5	6.8	A	0.18	7.0	16.1	A
	SB-L	0.14	22.0	20.3	C	0.22	22.6	19.5	C
	SB-T	0.55	41.5	131.4	D	0.53	41.2	106.1	D
	SB-R	0.04	8.3	2.5	A	0.07	11.6	9.6	B
Overall	0.55	29.8	-	C	0.62	28.9	-	C	
Sidney St / Kempton Ave (stop control)	WB-LR	0.15	10.8	4.2	B	0.11	12.3	2.8	B
	NB-TR	0.08	0.0	0.0	A	0.22	0.0	0.0	A
	SB-TL	0.01	0.6	0.2	A	0.02	1.5	0.5	A

Table 7: Intersection Capacity, Total Volumes, 2027.



		Intersection Capacity, Total Volumes, 2032							
		AM Peak Hour				PM Peak Hour			
		V/C	Delay (s)	Q ₉₅ (m)	LOS	V/C	Delay (s)	Q ₉₅ (m)	LOS
Highway 62 / Farnham Rd / Vermilyea Rd / (signalized)	EB-L	0.17	26.7	22.9	C	0.36	29.8	47.4	C
	EB-T	0.08	25.0	15.8	C	0.06	24.7	13.6	C
	EB-R	0.13	4.1	7.3	A	0.11	2.6	4.8	A
	WB-L	0.04	24.5	6.8	C	0.09	25.3	13.1	C
	WB-T	0.06	24.7	13.1	C	0.09	25.1	18.8	C
	WB-R	0.13	3.7	6.8	A	0.28	4.9	15.1	A
	NB-L	0.06	6.8	2.2	A	0.14	5.7	3.6	A
	NB-T	0.30	11.8	14.1	B	0.68	13.1	26.2	B
	NB-R	0.04	0.1	0.0	A	0.04	0.1	0.0	A
	SB-L	0.25	13.9	24.3	B	0.20	13.7	12.1	B
	SB-T	0.68	31.2	114.1	C	0.44	26.1	66.0	C
	SB-R	0.21	5.5	13.8	A	0.14	4.6	8.9	A
	Overall	0.68	21.3	-	C	0.68	16.8	-	B
Highway 62 / Roy Blvd (stop control)	EB-LR	0.31	18.2	10.5	C	0.47	20.5	19.7	C
	NB-L	0.14	11.1	3.9	B	0.17	10.3	5.0	B
	NB-T	0.14	0.0	0.0	A	0.30	0.0	0.0	A
	SB-T	0.37	0.0	0.0	A	0.29	0.0	0.0	A
	SB-R	0.21	0.0	0.0	A	0.18	0.0	0.0	A
Highway 62 / Maitland Dr (signalized)	EB-L	0.10	25.4	14.0	C	0.17	26.5	21.2	C
	EB-TR	0.39	19.9	49.2	B	0.32	24.2	47.9	C
	WB-L	0.54	36.4	57.3	D	0.56	36.1	63.8	D
	WB-TR	0.31	23.9	44.5	C	0.31	22.0	42.3	C
	NB-L	0.16	13.2	10.0	B	0.39	15.9	23.7	B
	NB-T	0.38	22.9	60.4	C	0.66	28.2	126.6	C
	NB-R	0.11	4.0	7.8	A	0.19	7.7	18.0	A
	SB-L	0.15	21.6	20.5	C	0.26	23.0	20.8	C
	SB-T	0.62	44.5	140.4	D	0.57	42.1	113.8	D
	SB-R	0.04	8.5	2.7	A	0.07	12.0	10.4	B
Overall	0.62	31.5	-	C	0.66	30.0	-	C	
Sidney St / Kempton Ave (stop control)	WB-LR	0.16	11.0	4.6	B	0.12	12.7	3.1	B
	NB-TR	0.08	0.0	0.0	A	0.23	0.0	0.0	A
	SB-TL	0.01	0.6	0.2	A	0.02	1.4	0.5	A

Table 8: Intersection Capacity, Total Volumes, 2032.



6 Auxiliary Lane Review

The need for auxiliary lanes (right turn taper and left turn lane) on Hwy 62 and Sidney St was reviewed for the intersections with Roy Blvd and Kempton Ave, respectively.

The Ministry of Transportation Ontario Design Supplement for the TAC Manual was consulted to evaluate the need for left turn lanes. The methodology for two-lane highways is based on the volumes advancing with the left turns, the proportion of left turns, and the volumes opposing the left turns.

The existing two-way left turn lane serves left-turning vehicles entering Roy Blvd from Hwy 62, so a review is not needed for this movement. For left-turning vehicles entering Kempton Ave from Sidney St, the results show that a left turn lane is not warranted (see *Appendix J* for further details).

According to the TAC Manual, a right turn volume warrant of 60 vehicles per hour is often used as indicator that a more detailed capacity analysis of the intersection and geometric options for accommodating right turning traffic is required.

The right-turning volumes entering Roy Blvd from Hwy 62 are not expected to reach the criteria, with 51 veh/h during the 2032 PM peak hour for the total traffic volumes scenario.

It is observed that the right-turning volumes from Sidney St into Kempton Ave are expected to reach 77 veh/h during the 2027 PM peak hour for the background traffic volumes scenario. By reviewing the capacity of the shared northbound thru/right movement, it is noticed that although the right turn volumes are slightly over 60 vehicles per hour, the capacity is at the best thus it is determined that a right turn taper is not currently warranted.



7 Conclusions/Recommendations

As discussed throughout the report, a residential subdivision with a small commercial area is proposed adjacent to Highway 62 in Northern Belleville, bounded between a recent subdivision on the west and an established commercial area on the east.

Traffic movement counts were provided by MTO for the intersections of Hwy 62 at Farnham Rd, Roy Blvd, and Maitland Dr. A video-based count was performed by Asurza Engineers to gain data for Sidney St / Kempton Ave.

For the existing conditions (2024), results show that the intersections are currently operating with acceptable control delays during peak hours. Almost all movements have a LOS “C” or better, and all movements are showing v/c ratios well below 0.85.

In order to establish base conditions for the comparison and evaluation of future scenarios, a 1.5% growth rate was used to project the traffic volumes throughout the study years.

The results show that, without the proposed development, delays will moderately increase over time as expected. Throughout the study years, all movements will continue to operate with acceptable levels of service.

The estimation of trips generated by the proposed development were derived from the Trip Generation Manual, 11th Edition, published by ITE. The proposed development is estimated to generate 112 and 137 new trips during the AM and PM peak hours, respectively.

The future total traffic volumes are obtained by adding the background traffic volumes to the total new traffic volumes generated by the proposed development.



The results show that, including the proposed development, the delays and LOS for all movements remain basically the same as the background scenario. The most critical movements are expected to be the westbound-left and southbound-thru movements at Highway 62 / Maitland Dr, which both show LOS “D” by 2032 regardless of the proposed development. Also, the v/c ratio for all movements will remain well below 0.85 throughout the study years.

It was also determined that auxiliary lanes (right turn taper or left turn lanes) are not currently warranted at the unsignalized intersections Hwy 62/Roy Blvd. and Sidney St/Kempton Ave. Results on the turning movement operations show that there is enough capacity to manage additional traffic volumes.

Therefore, from the traffic point of view, it is concluded that the proposed residential development can occur without any improvements to the existing geometric and traffic control conditions.



Appendix A

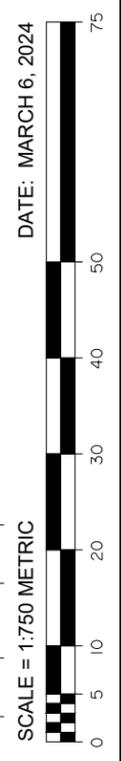
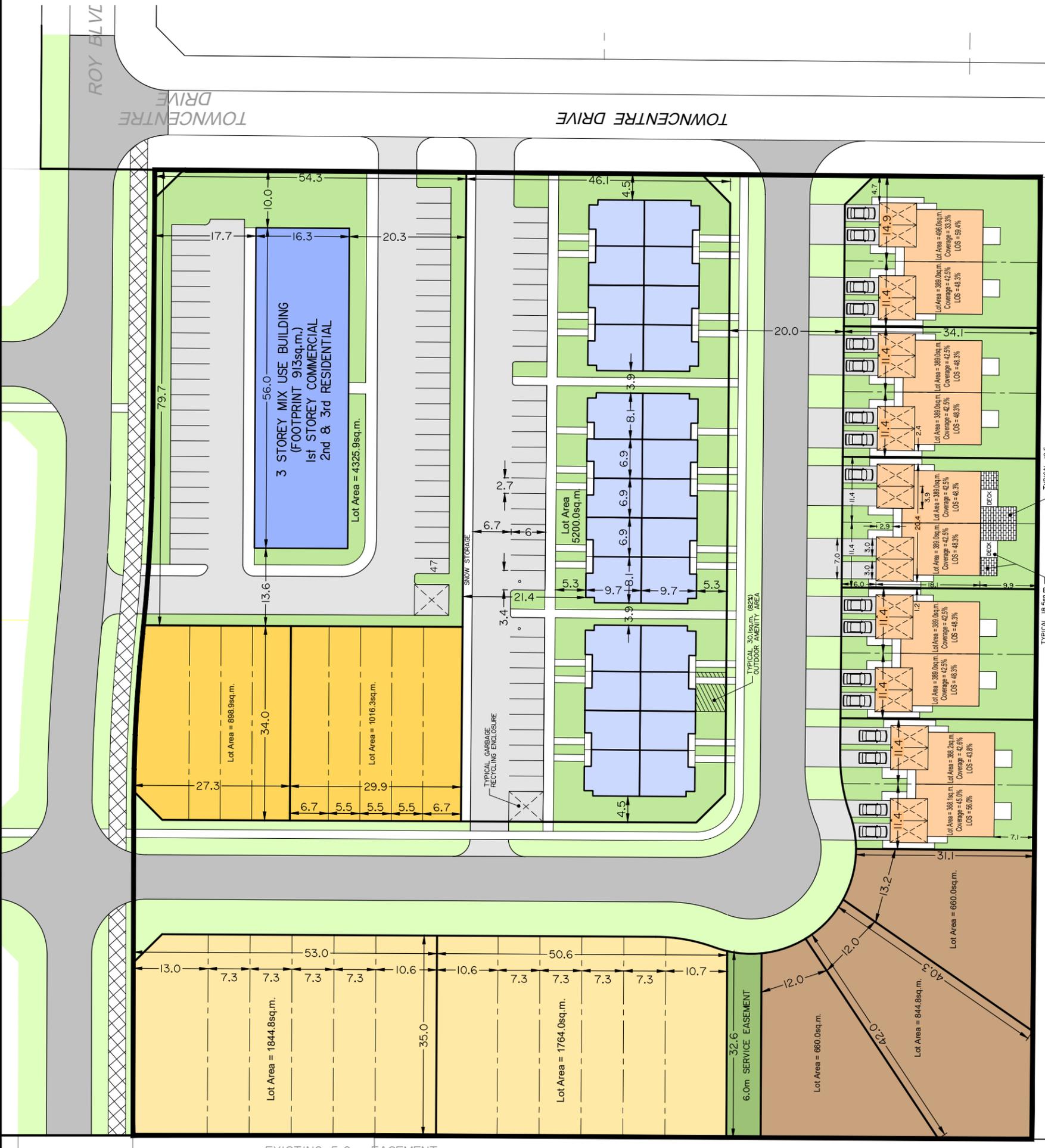
Preliminary Concept Plan and
Draft Plan of Subdivision

TOWNCENTRE PLACE - CONCEPT

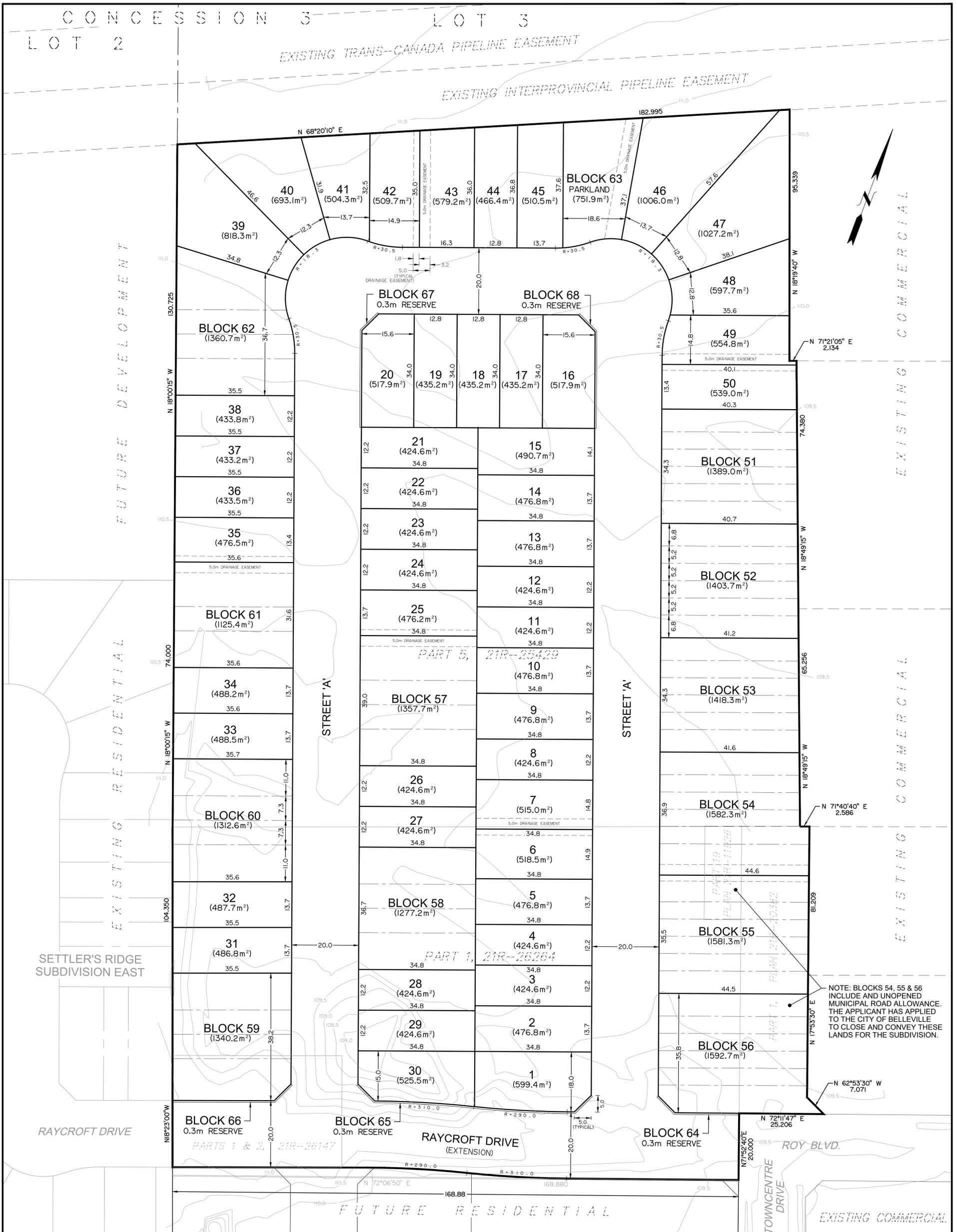
PART 5, REGISTERED PLAN 21R-25429,
PART OF PART 9,
REGISTERED PLAN 21R-11926
PART OF LOT 3, CONCESSION 3

(FORMERLY THURLOW TOWNSHIP)
CITY OF BELLEVILLE
COUNTY OF HASTINGS

LAND USE	PROPOSED CONCEPT
7.3m (MIN.) BUNGALOW TOWNHOUSE	12
5.5m (MIN.) 2 STOREY TOWNHOUSE	9
12.0m (MIN.) TWO-UNIT DWELLING	6
11.4m (MIN.) SEMI-DETACHED DWELLING WITH A.D.U.	20
6.9m (MIN.) BACK-TO-BACK TOWNHOUSES	26
3 STOREY MIX USE BUILDING	20
6.0m SERVICE EASEMENT	
20.0m ROAD MUNICIPAL ROAD ALLOWANCE	
SITE TOTAL	93



211 Dundas Street East, Suite 202,
Belleville, Ontario

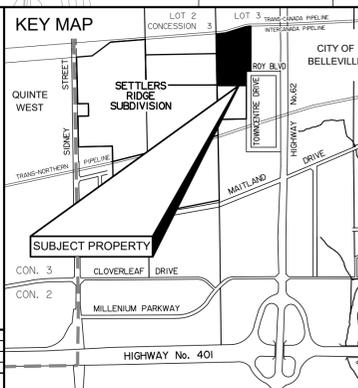


DRAFT PLAN OF SUBDIVISION
SETTLERS RIDGE EAST
 PART 5, REGISTERED PLAN 21R-25429,
 PART 1, REGISTERED PLAN 21R-26264,
 PART 9, REGISTERED PLAN 21R-11926,
 PART 1, REGISTERED PLAN 21R-20382,
 PARTS 1&2, REGISTERED PLAN 21R-26147
 PART OF LOT 3, CONCESSION 3

GEOGRAPHIC TOWNSHIP OF THURLOW,
 NOW IN THE
CITY OF BELLEVILLE,
 COUNTY OF HASTINGS

SCALE 1:500

1st SUBMISSION TO THE CITY OF BELLEVILLE
 DATE: 02/29/24
 DRAWN BY: LB CHECKED BY: R.F.A. DATE: 02/29/24



LAND USE SCHEDULE

LAND USE	AREA(m ²)	AREA% UNITS
LOTS 1, 2, 5, 6, 9, 10, 13-15, 31-34, 41-43 & 46 - 13.7m (476.8m ²) MIN. - SINGLE-DETACHED LOTS	9530.3	16.4 18
LOTS 3, 4, 7, 8, 11, 12, 16-30, 35-40, 44 & 47-50 - 12.2m (424.6m ²) MIN. - SINGLE-DETACHED LOTS	15851.4	27.2 32
BLOCKS 51-56 - 5.2m (209.4m ²) MIN. - FREEHOLD 2 STOREY TOWNHOUSE UNITS	8967.3	15.4 36
BLOCKS 57-62 - 7.3m (254.6m ²) MIN. - FREEHOLD BUNGALOW TOWNHOUSE UNITS	7773.8	13.4 23
BLOCK 63 - PARKLAND	751.9	1.3
BLOCKS 64-68 - 0.3m RESERVES	0.0	0.0
20.0m MUNICIPAL ROAD ALLOWANCE - STREETS 'A' AND RAYCROFT DRIVE EXTENSION (LENGTH = 366.6m)	15288.9	26.3
SITE TOTAL	58163.6m²	100.0% 109

METRIC NOTE:
 DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048.

CONTOURS NOTE:
 CONTOURS PREPARED USING DATA OBTAINED BY OTHERS. CONTOURS DRAWN AT INTERVALS OF 0.5m.

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51(17) OF THE PLANNING ACT.

- SEE SURVEYOR'S CERTIFICATE.
- AS SHOWN ON DRAFT PLAN.
- SEE LAND USE SUMMARY.
- SEE DRAFT PLAN.
- AS SHOWN ON DRAFT PLAN.
- AS SHOWN ON DRAFT PLAN.
- MUNICIPAL WATER AND SANITARY SEWER.
- SHALLOW LOAM OVER BEDROCK.
- AS SHOWN ON DRAFT PLAN.
- GARBAGE COLLECTION, FIRE PROTECTION, ROAD MAINTENANCE, SCHOOL BUSES, ETC.
- AS SHOWN ON DRAFT PLAN.

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SURVEYOR'S CERTIFICATE
 I CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ARE CORRECTLY SHOWN.

KEITH WATSON, ONTARIO LAND SURVEYOR. DATE: _____

WATSON LAND SURVEYORS LTD.
 215 CHURCH STREET, BELLEVILLE, ONTARIO
 (613) 962-9521

FEBRUARY 29, 2024

RFA PLANNING CONSULTANT INC.

211 Dundas Street East, Suite 202, Belleville, Ontario, K0N 1Z2

691-DP

Appendix B

Traffic Data



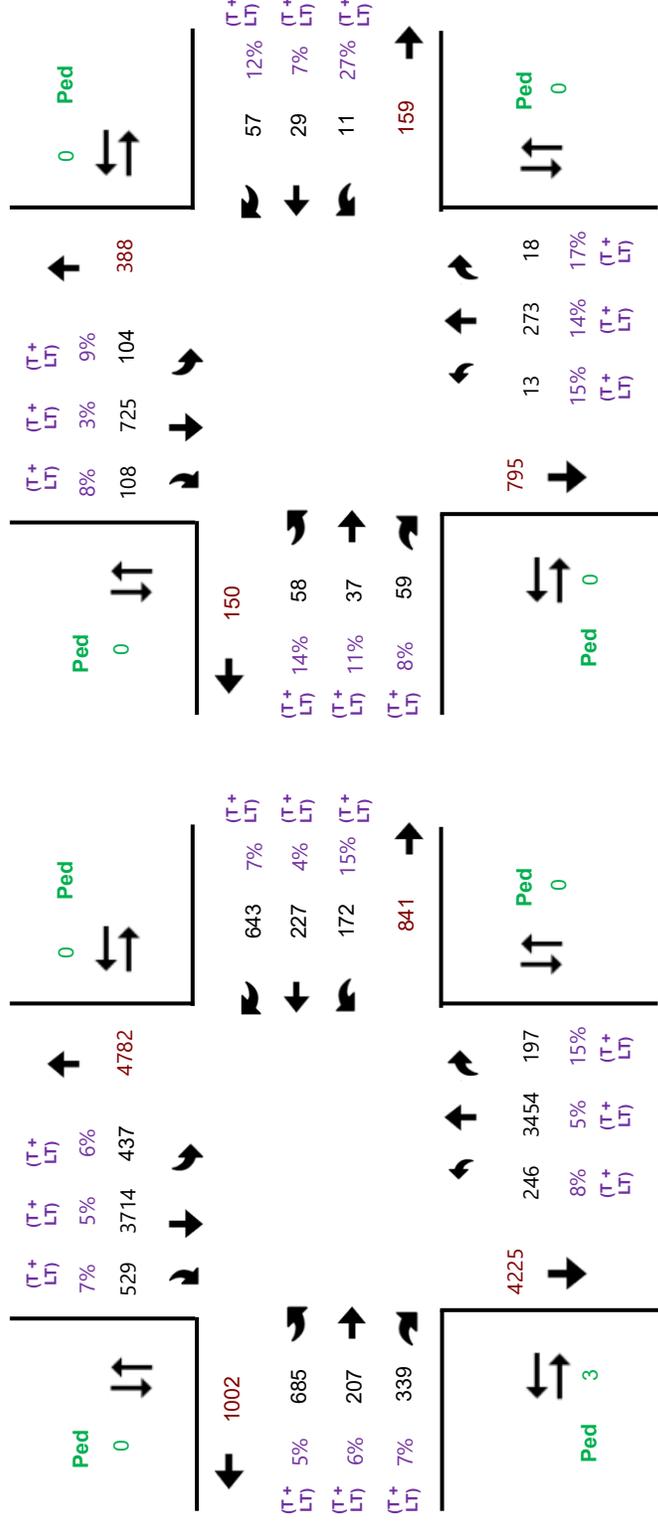
TES - Traffic Engineering System

Turning Movement Total Count and Peak Summary Report

Description: HWY 62 @ FARNHAM RD_VERMILYEA RD
Region: EAST **Hwy #:** HWY 62
LHRS_Offset: 33690_0304 **Int. Type:** Cross
Count Date: Tuesday, 10 May, 2022

Full Study Tue, 10 May, 2022 07:00 AM - 06:00 PM

AM Period Tue, 10 May, 2022 07:15 AM - 08:15 AM





TES - Traffic Engineering System

Turning Movement Total Count and Peak Summary Report

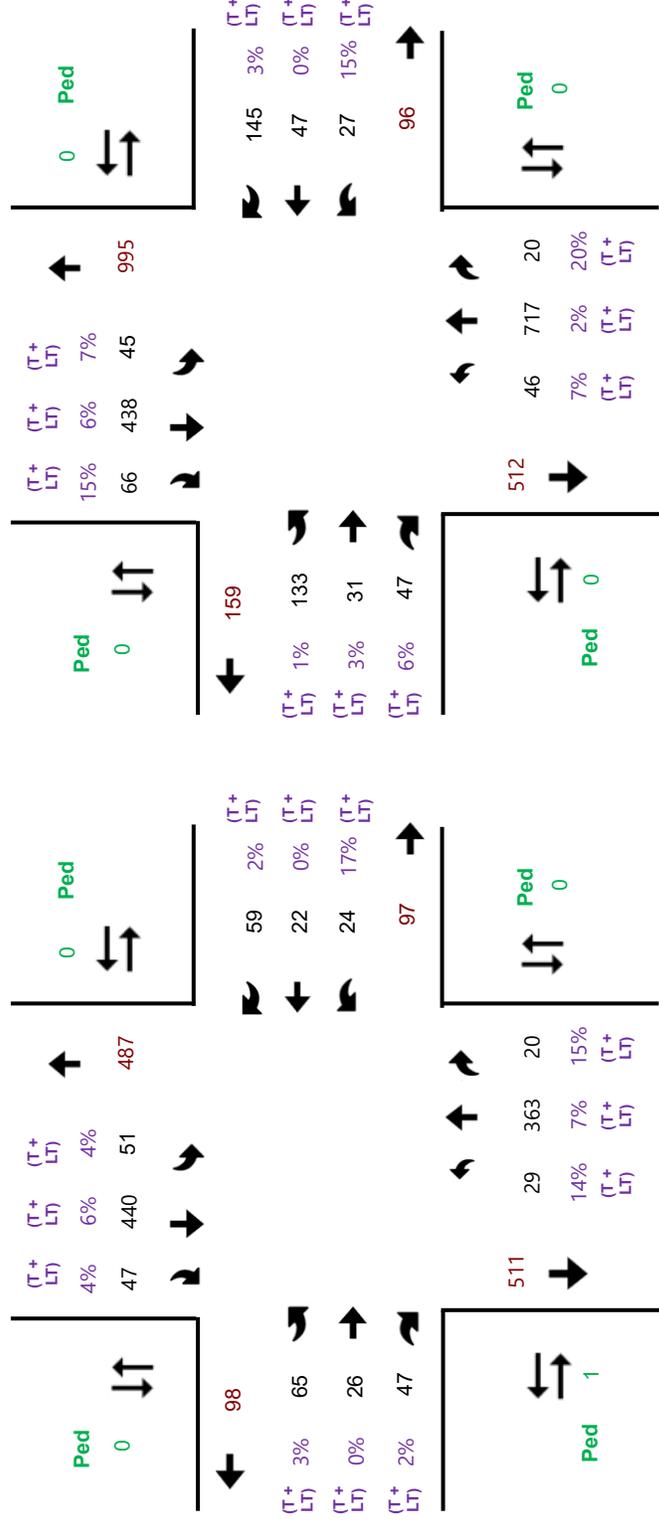
Description: HWY 62 @ FARNHAM RD_VERMILYEA RD
Region: EAST
LHRS_Offset: 33690_0304
Count Date: Tuesday, 10 May, 2022
Hwy #: HWY 62
Int. Type: Cross

PM Period

Tue, 10 May, 2022 04:15 PM - 05:15 PM

MD Period

Tue, 10 May, 2022 12:30 PM - 01:30 PM





TES - Traffic Engineering System

Turning Movement Total Count and Peak Summary Report

Description: HWY 62 @ ROY BLVD
Region: EAST
LHRS_Offset: 33690_0155
Count Date: Tuesday, 10 May, 2022

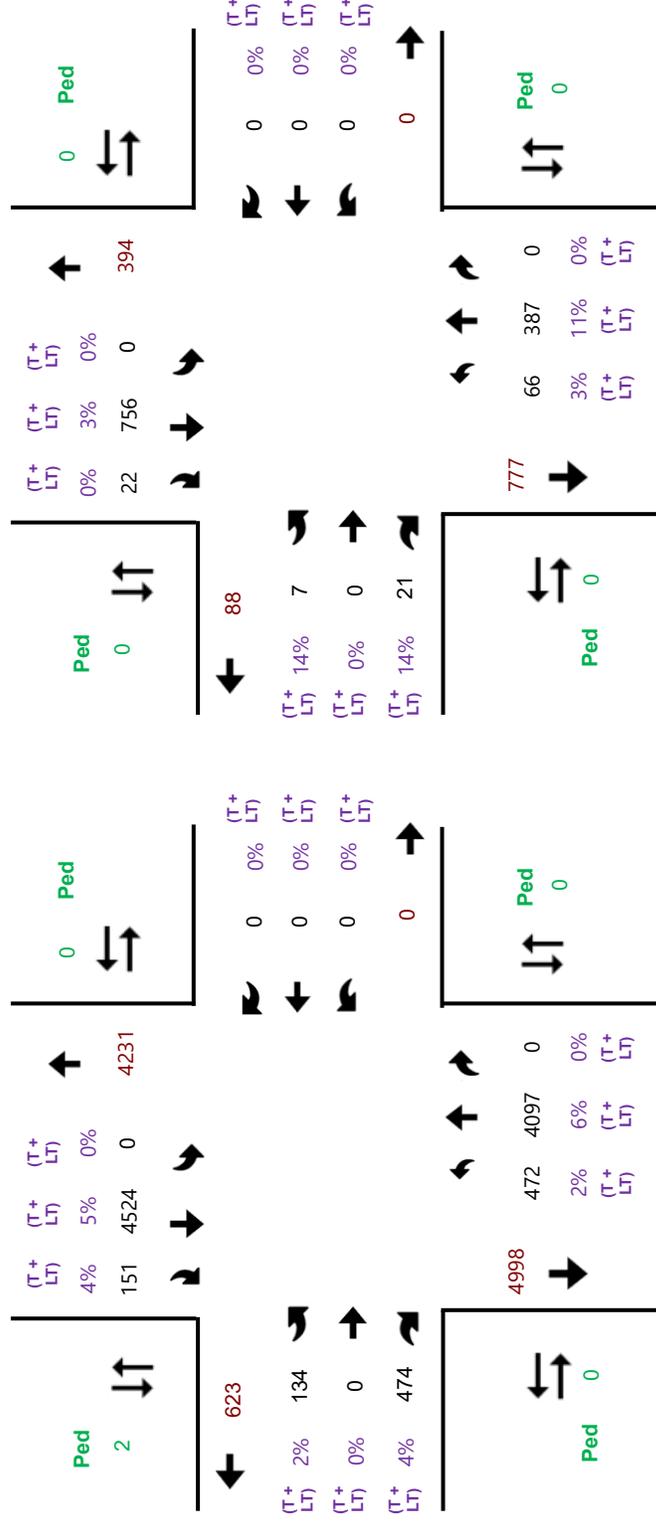
Hwy #: HWY 62
Int. Type: T-Intersection

Full Study

Tue, 10 May, 2022 07:00 AM - 06:00 PM

AM Period

Tue, 10 May, 2022 07:30 AM - 08:30 AM





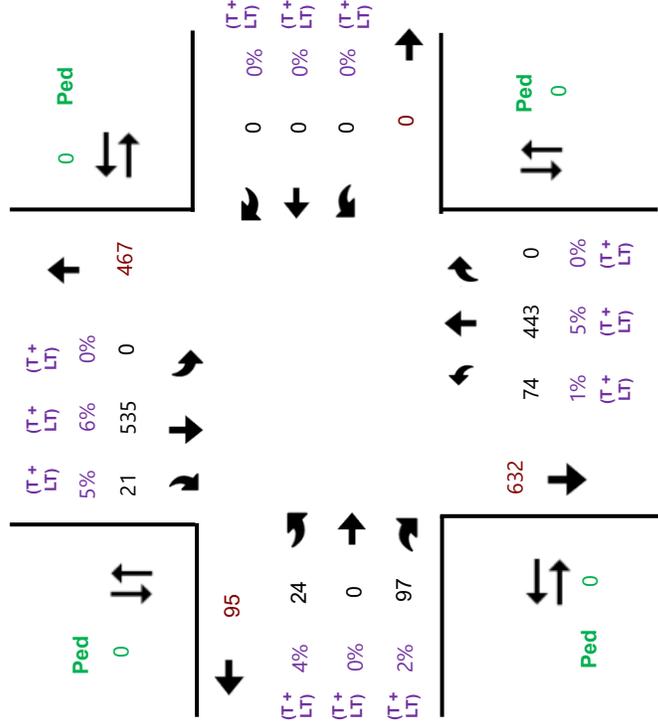
TES - Traffic Engineering System

Turning Movement Total Count and Peak Summary Report

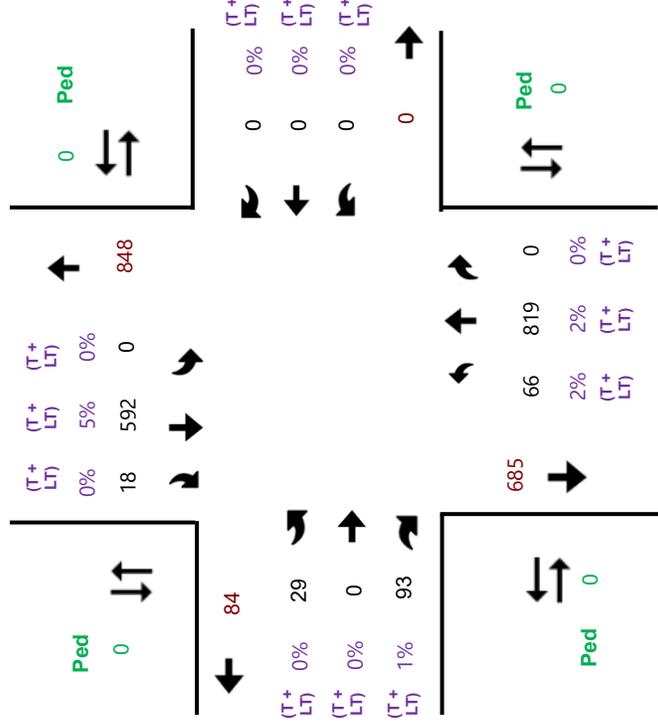
Description: HWY 62 @ ROY BLVD
Region: EAST
LHRS_Offset: 33690_0155
Count Date: Tuesday, 10 May, 2022

Hwy #: HWY 62
Int. Type: T-Intersection

MD Period Tue, 10 May, 2022 12:00 PM - 01:00 PM



PM Period Tue, 10 May, 2022 04:30 PM - 05:30 PM





TES - Traffic Engineering System

Turning Movement Total Count and Peak Summary Report

Description: HWY 62 @ MAITLAND DR

Region: EAST

Hwy #: HWY 62

LHRS_Offset: 33690_0104

Int. Type: Cross

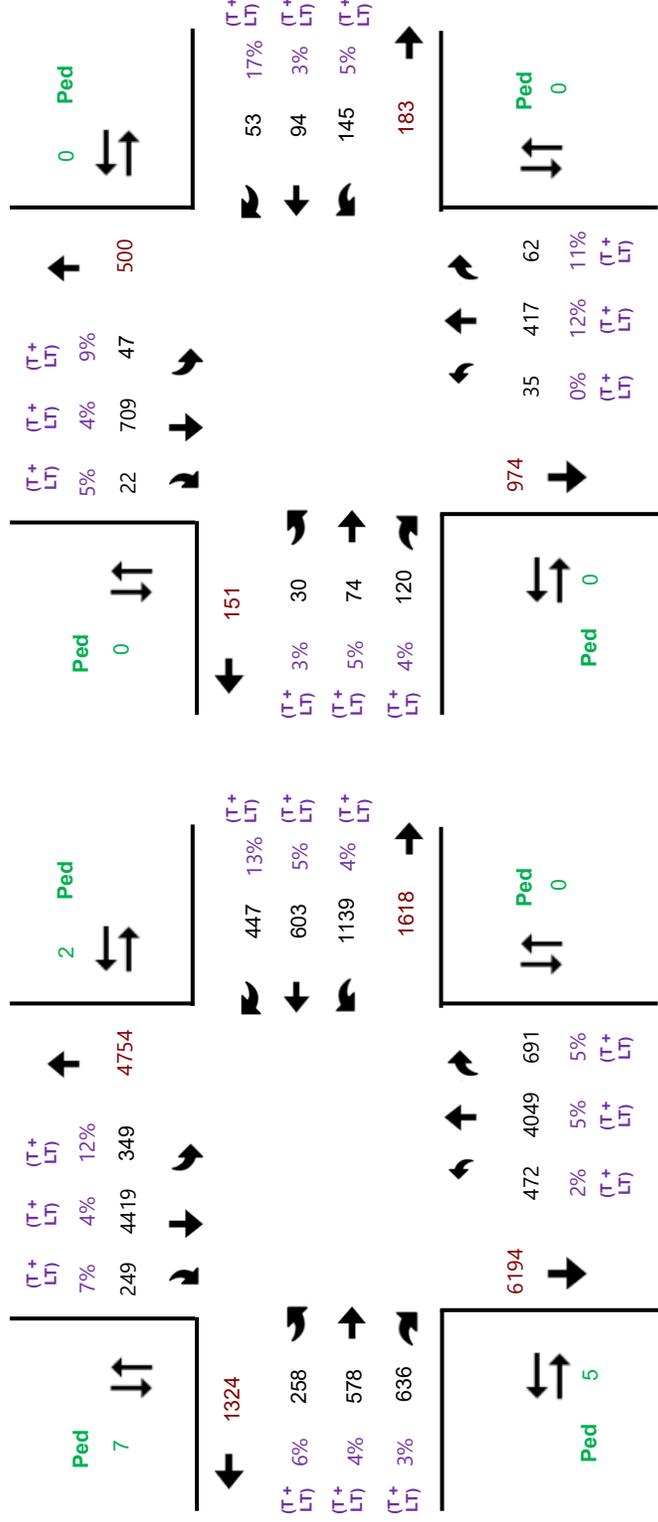
Count Date: Tuesday, 10 May, 2022

Full Study

Tue, 10 May, 2022 07:00 AM - 06:00 PM

AM Period

Tue, 10 May, 2022 07:30 AM - 08:30 AM



Appendix C

Existing Signal Timings

ACTUATED INTERVAL TIMING AND FAZE FUNCTIONS

	PHASE							
	1	2	3	4	5	6	7	8
0 Ped Walk	-	7	-	7	-	7	-	7
1 Ped FDW	-	31	-	36	-	31	-	36
2 Min Green	7	20	-	15	7	20	-	15
3 Type 3 Discon	-	-	-	-	-	-	-	-
4 Added per Veh	-	-	-	-	-	-	-	-
5 Veh Extension	3.0	4.5	-	3.0	3.0	4.5	-	3.0
6 Max Gap	3.0	4.5	-	3.0	3.0	4.5	-	3.0
7 Min Gap	3.0	4.5	-	3.0	3.0	4.5	-	3.0
8 Max Limit	15	50	-	45	15	50	-	45
9 Max Limit 2	-	-	-	-	-	-	-	-
A Adv/Dly Walk	-	-	-	-	-	-	-	-
B PE Min Ped FDW	-	-	-	-	-	-	-	-
C Cond Serv Check	-	-	-	-	-	-	-	-
D Reduce Every	-	-	-	-	-	-	-	-
E Yellow Change	3.0	5.9	-	5.0	3.0	5.9	-	5.0
F Red Clear	2.0	1.4	-	2.2	2.0	1.4	-	2.2

PHASE BANK #1 < C + O + F = 1 >

	9	A	B	C	D	E
	0					
1 Phase 1	-					RR-1 Clear
2 Phase 2	-					EV-A Delay
3 Phase 3	-					EV-A Clear
4 Phase 4	-					EV-B Delay
5 Phase 5	-					EV-B Clear
6 Phase 6	-					EV-C Delay
7 Phase 7	-					EV-C Clear
8 Phase 8	-					EV-D Delay
		MAX ALT	ALT	ALT	ALT	EV-D Clear
		INT WALK	FLH	INT	EXT	RR-2 Delay
		D/W				RR-2 Clear
						View EV Delay
						View EV Clear
						View RR Delay
						View RR Clear

ALL RED START

$(F/1 + C + O) =$

5.0

RED REVERT

$(F/1 + O + F) =$

5.0

	COLUMN F PHASES							
	1	2	3	4	5	6	7	8
0 Permit	X	X						X
1 Red Lock				X	X	X	X	X
2 Yellow Lock								
3 Min Recall				X				X
4 Ped Recall								
5 View Set Peds								
6 Rest in Walk								
7 Red Rest								
8 Dual Entry				X	X	X	X	X
9 Max Recall								
A Soft Recall								
B Max 2								
C Cond. Service								
D Man Ctrl Calls								
E Yellow Start				X				X
F First Phases				X	X	X	X	X

< C + O + F = 1 >

BI Tran Systems, Inc.
510 Bearcat Dr., Sacramento, Calif. 95814
Traffic Signal Program **233** Ontario
Timing Sheet #2

Date: 25-Mar-13

LOCATION
Hwy: 62
At:

Farnhma/Vermilyea Rd

PREEMPT	A	B	C
RR1-2	SP	EMER	
MINIMUMS	SPEV1	EV2	VEH
A WLK (DFLT)	4.0	4.0	4.0
B FD WALK			
C INITIAL			

< C + O + F = 1 >

	Column E Phases / Bits							
	1	2	3	4	5	6	7	8
0 Exclusive								
1 RR-1 Clear								
2 RR-2 Clear								
3 RR-2 Ltd Srv								
4 Prot/Perm								
5 Flash to PE								
6 Flash Entry								
7 Disable Yellow								
8 Disable Ovp Yel								
9 Ovp Yellow FLH								
A EV-A Phases								
B EV-B Phases								
C EV-C Phases								
D EV-D Phases								
E Extra 1	X					X		
F IC Select	X							

< C + O + E = 125 >

- FLASH TO PREEMPT**
- 1 = EVA
 - 2 = EVB
 - 3 = EVC
 - 4 = EVD
 - 5 = RR1
 - 6 = RR2
 - 7 = SE1
 - 8 = SE2
 - 1 = TBC TYPE 1
 - 2 = NEMA EXT. COORD.
 - 3 = DAYLIGHT SAVINGS
 - 4 =
 - 5 = EXPANDED STATUS REPORTING
 - 6 = INTERNATIONAL PED
 - 7 = CLEAR OUTPUTS DURING FLASH
 - 8 = SPLIT RING

	Column F Phases / Bits							
	1	2	3	4	5	6	7	8
0								
1 Ext. Permit 1 Phases								
2 Ext. Permit 2 Phases								
3 Exclusive Ped Assign								
4 Preempt Non-Lock								
5 Ped for 2P Output	X							
6 Ped for 6P Output						X		
7 Ped for 4P Output				X				
8 Ped for 8P Output								X
9 Yellow Flashes Phases								
A Low Priority A Phases								
B Low Priority B Phases								
C Low Priority C Phases								
D Low Priority D Phases								
E Restricted Phases								
F Extra 2 Config. Bits								

- EXTRA 1**
- 5 = EXPANDED STATUS REPORTING
 - 6 = INTERNATIONAL PED
 - 7 = CLEAR OUTPUTS DURING FLASH
 - 8 = SPLIT RING

	Column F Phases / Bits							
	1	2	3	4	5	6	7	8
0 Fast Green Flash Phase								
1 Green Flash Phases								
2 Flashing Walk Phases								
3 Guaranteed Passage								
4 Simultaneous Gap Term				X				X
5 Sequential Timing								
6 Advance Walk Phases								
7 Delay Walk Phases								
8 External Recall								
9 Start-up Overlap Green								
A Max Extension								
B Inhibit Ped Reserve/ice								
C Semi-Actuated								
D Start-up Overlap Yellow								
E Start-up Vehicle Calls				X	X	X	X	X
F Start-up Ped Calls				X	X	X	X	X

SPECIALS < C + O + F = 2 >

- EXTRA 2**
- 1 = AWR ON DURING PHASE INITIAL
 - 2 = LMU INSTALLED
 - 2 = 2 WAY MODEM
 - 3 = 7 WIRE SLAVE
 - 4 = FLASH / FREE
 - 5 = SIMPLEX MASTER
 - 7 = 7 WIRE MASTER
 - 8 = OFFSET INTURP

MANUAL PLAN
< C/O + A + 1 >
MANUAL OFFSET
< C/O + B + 1 >
MANUAL SELECTION

- MANUAL PLAN**
- 0 = Automatic (Master)
 - 9 = Control Plan 1 - 9
 - 14 (E) = Free (Isolated)
 - 15 (F) = Software Flash

MANUAL OFFSET

- 0 = Automatic (Master)
- 1 = Offset A
- 2 = Offset B
- 3 = Offset C

ACTUATED INTERVAL TIMING AND FAZE FUNCTIONS

	PHASE							
	1	2	3	4	5	6	7	8
0 Ped Walk	-	7	-	7	-	7	-	7
1 Ped FDW	-	26	-	34	-	26	-	34
2 Min Green	7	20	-	10	7	20	-	10
3 Type 3 Discon	-	-	-	-	-	-	-	-
4 Added per Veh	-	-	-	-	-	-	-	-
5 Veh Extension	3.0	3.6	-	3.0	3.0	3.6	-	3.0
6 Max Gap	3.0	3.6	-	3.0	3.0	3.6	-	3.0
7 Min Gap	3.0	3.6	-	3.0	3.0	3.6	-	3.0
8 Max Limit	15	50	-	45	15	50	-	45
9 Max Limit 2	-	-	-	-	-	-	-	-
A Adv/Dly Walk	-	-	-	-	-	-	-	-
B PE Min Ped FDW	-	-	-	-	-	-	-	-
C Cond Serv Check	-	-	-	-	-	-	-	-
D Reduce Every	-	-	-	-	-	-	-	-
E Yellow Change	3.0	5.0	-	4.5	3.0	5.0	-	4.5
F Red Clear	2.0	1.7	-	2.5	2.0	1.7	-	2.5

PHASE BANK #1 < C + O + F = 1 >

	9	A	B	C	D	E
	0					
1 Phase 1	-					RR-1 Clear
2 Phase 2	-					EV-A Delay
3 Phase 3	-					EV-A Clear
4 Phase 4	-					EV-B Delay
5 Phase 5	-					EV-B Clear
6 Phase 6	-					EV-C Delay
7 Phase 7	-					EV-C Clear
8 Phase 8	-					EV-D Delay
		MAX ALT	ALT	ALT	ALT	EV-D Clear
		INT WALK	FLH	INT	EXT	RR-2 Delay
		D/W				RR-2 Clear
						View EV Delay
						View EV Clear
						View RR Delay
						View RR Clear

ALL RED START
 (F/1 + C + O) = **5.0**
 RED REVERT
 (F/1 + O + F) = **5.0**

	COLUMN F PHASES							
	1	2	3	4	5	6	7	8
0 Permit	X	X						X
1 Red Lock				X	X			
2 Yellow Lock								
3 Min Recall			X			X		
4 Ped Recall								
5 View Set Peds								
6 Rest in Walk								
7 Red Rest								
8 Dual Entry		X		X		X		X
9 Max Recall								
A Soft Recall								
B Max 2								
C Cond. Service								
D Man Ctrl Calls								
E Yellow Start				X			X	
F First Phases				X				X

< C + O + F = 1 >

BI Tran Systems, Inc.
 510 Bearcat Dr., Sacramento, Calif. 95814
 Traffic Signal Program **233** Ontario
 Timing Sheet #2

Date: 14-Jun-13
LOCATION
 Hwy: 62
 At: Maitland Dr

PREEMPT	A	B	C
	RR1-2	SP	EMER
MINIMUMS	SPEV1	EV2	VEH
A WLK (DFLT)	4.0	4.0	4.0
B FD WALK			
C INITIAL			

< C + O + F = 1 >

	Column E Phases / Bits							
	1	2	3	4	5	6	7	8
0 Exclusive								
1 RR-1 Clear								
2 RR-2 Clear								
3 RR-2 Ltd Srv								
4 Prot/Perm								
5 Flash to PE								
6 Flash Entry								
7 Disable Yellow								
8 Disable Ovp Yel								
9 Ovp Yellow FLH								
A EV-A Phases								
B EV-B Phases								
C EV-C Phases								
D EV-D Phases								
E Extra 1	X		X		X		X	
F IC Select	X							

< C + O + E = 125 >

	Column F Phases / Bits							
	1	2	3	4	5	6	7	8
0								
1 Ext. Permit 1 Phases								
2 Ext. Permit 2 Phases								
3 Exclusive Ped Assign								
4 Preempt Non-Lock								
5 Ped for 2P Output	X							
6 Ped for 6P Output						X		
7 Ped for 4P Output				X				
8 Ped for 8P Output								X
9 Yellow Flashes Phases								
A Low Priority A Phases								
B Low Priority B Phases								
C Low Priority C Phases								
D Low Priority D Phases								
E Restricted Phases								
F Extra 2 Config. Bits								

< C + O + E = 125 >

	Column F Phases / Bits							
	1	2	3	4	5	6	7	8
0 Fast Green Flash Phase								
1 Green Flash Phases								
2 Flashing Walk Phases								
3 Guaranteed Passage								
4 Simultaneous Gap Term	X			X		X		X
5 Sequential Timing								
6 Advance Walk Phases								
7 Delay Walk Phases								
8 External Recall								
9 Start-up Overlap Green								
A Max Extension								
B Inhibit Ped Reserve/ice								
C Semi-Actuated								
D Start-up Overlap Yellow								
E Start-up Vehicle Calls	X	X		X	X	X	X	X
F Start-up Ped Calls	X	X		X	X	X	X	X

SPECIALS < C + O + F = 2 >

- FLASH TO PREEMPT**
 1 = EVA
 2 = EVB
 3 = EVC
 4 = EVD
 5 = RR1
 6 = RR2
 7 = SE1
 8 = SE2
 1 = TBC TYPE 1
 2 = NEMA EXT. COORD.
 3 = DAYLIGHT SAVINGS
 4 =
 5 = EXPANDED STATUS REPORTING
 6 = INTERNATIONAL PED
 7 = CLEAR OUTPUTS DURING FLASH
 8 = SPLIT RING

- EXTRA 2**
 1 = AWR ON DURING PHASE INITIAL
 2 = LMU INSTALLED
 2 = 2 WAY MODEM
 3 = 7 WIRE SLAVE
 4 = FLASH / FREE
IC SELECT
 5 = SIMPLEX MASTER
 7 = 7 WIRE MASTER
 8 = OFFSET INTURP

Appendix D

Traffic Volume Projections

Hwy 62 / Farnham Rd		Eastbound			Northbound			Westbound			Southbound			Volume Type
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
	AM	58	37	59	13	273	18	11	29	57	104	725	108	Registered Volumes
2022	PM	133	31	47	46	717	20	27	47	145	45	438	66	
Annual Growth Factor 1.50%														
	AM	60	38	61	13	281	19	11	30	59	107	747	111	Existing Volumes 2024
2024	PM	137	32	48	47	739	21	28	48	149	46	451	68	
	AM	62	40	64	14	294	19	12	31	61	112	781	116	Background Volumes 2027
2027	PM	143	33	51	50	772	22	29	51	156	48	472	71	
	AM	67	43	68	15	317	21	13	34	66	121	841	125	Background Volumes 2032
2032	PM	154	36	55	53	832	23	31	55	168	52	508	77	
	AM	0	0	1	1	19	2	0	0	0	0	14	0	Subject Site Generated Trips
2027	PM	0	0	2	1	23	0	2	0	0	0	24	0	
	AM	62	40	65	15	313	21	12	31	61	112	795	116	Total Volumes 2027
2027	PM	143	33	53	51	795	22	31	51	156	48	496	71	
	AM	67	43	69	16	336	23	13	34	66	121	855	125	Total Volumes 2032
2032	PM	154	36	57	54	855	23	33	55	168	52	532	77	

Hwy 62 / Roy Blvd		Eastbound			Northbound			Westbound			Southbound			Volume Type	
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
	AM	7	0	21	66	387	0	0	0	0	0	0	756	22	Registered Volumes
2022	PM	29	0	93	66	819	0	0	0	0	0	0	592	18	
Annual Growth Factor 1.50%															
	AM	7	0	22	68	399	0	0	0	0	0	0	779	23	Existing Volumes 2024
2024	PM	30	0	96	68	844	0	0	0	0	0	0	610	19	
	AM	8	0	23	71	417	0	0	0	0	0	0	814	24	Background Volumes 2027
2027	PM	31	0	100	71	882	0	0	0	0	0	0	638	19	
	AM	8	0	24	77	449	0	0	0	0	0	0	877	26	Background Volumes 2032
2032	PM	34	0	108	77	950	0	0	0	0	0	0	687	21	
	AM	22	0	55	10	0	0	0	0	0	0	0	0	15	Subject Site Generated Trips
2027	PM	24	0	21	50	0	0	0	0	0	0	0	0	28	
	AM	30	0	78	81	417	0	0	0	0	0	0	814	39	Total Volumes 2027
2027	PM	55	0	121	121	882	0	0	0	0	0	0	638	47	
	AM	30	0	79	87	449	0	0	0	0	0	0	877	41	Total Volumes 2032
2032	PM	58	0	129	127	950	0	0	0	0	0	0	687	49	

Hwy 62 / Maitland Blvd		Eastbound			Northbound			Westbound			Southbound			Volume Type	
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
	2022	AM	30	74	120	35	417	62	145	94	53	47	709	22	Registered Volumes
		PM	50	102	59	775	110	166	82	63	63	44	614	39	
Annual Growth Factor 1.50%															
	2024	AM	31	76	124	36	430	64	149	97	55	48	730	23	Existing Volumes 2024
		PM	52	105	61	106	798	113	171	84	65	65	45	633	
	2027	AM	32	80	129	38	449	67	156	101	57	51	764	24	Background Volumes 2027
		PM	54	110	64	111	835	119	179	88	68	47	661	42	
	2032	AM	35	86	139	41	484	72	168	109	62	55	823	26	Background Volumes 2032
		PM	58	118	68	120	899	128	193	95	73	51	713	45	
	2027	AM	1	0	0	0	8	0	0	0	1	4	49	2	Subject Site Generated Trips
		PM	3	0	0	0	43	0	0	0	4	1	19	1	
	2027	AM	33	80	129	38	457	67	156	101	58	55	813	26	Total Volumes 2027
		PM	57	110	64	111	878	119	179	88	72	48	680	43	
	2032	AM	36	86	139	41	492	72	168	109	63	59	872	28	Total Volumes 2032
		PM	61	118	68	120	942	128	193	95	77	52	732	46	

Sidney St / Kempton Ave		Eastbound			Northbound			Westbound			Southbound			Volume Type	
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
	AM	0	0	0	0	83	25	55	0	0	10	8	131	0	Registered Volumes
	PM	0	0	0	0	247	49	25	0	5	13	108	0		
Annual Growth Factor 1.50%															
	AM	0	0	0	0	83	25	55	0	10	8	131	0	Existing Volumes 2024	
	PM	0	0	0	0	247	49	25	0	5	13	108	0		
	AM	0	0	0	0	0	8	25	0	5	2	0	0	Settler's Ridge East P2 Generated Trips	
	PM	0	0	0	0	0	26	17	0	3	7	0	0		
	AM	0	0	0	0	87	34	83	0	15	10	137	0	Background Volumes 2027	
	PM	0	0	0	0	258	77	43	0	8	21	113	0		
	AM	0	0	0	0	93	36	87	0	16	11	148	0	Background Volumes 2032	
	PM	0	0	0	0	278	81	45	0	9	22	122	0		
	AM	0	0	0	0	0	1	3	0	1	0	0	0	Subject Site Generated Trips	
	PM	0	0	0	0	0	3	2	0	0	1	0	0		
	AM	0	0	0	0	87	35	86	0	16	10	137	0	Total Volumes 2027	
	PM	0	0	0	0	258	80	45	0	8	22	113	0		
	AM	0	0	0	0	93	37	90	0	17	11	148	0	Total Volumes 2032	
	PM	0	0	0	0	278	84	47	0	9	23	122	0		

Appendix E

Synchro Reports,
Existing Volumes 2024

HCM Signalized Intersection Capacity Analysis

Existing Volumes 2024

1: Maitland Dr & Hwy 62

AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	31	76	124	149	97	55	36	430	64	48	730	23
Future Volume (vph)	31	76	124	149	97	55	36	430	64	48	730	23
Satd. Flow (prot)	1752	1651	0	1719	1661	0	1805	3223	1455	1656	3471	1538
Flt Permitted	0.648			0.574			0.277			0.435		
Satd. Flow (perm)	1195	1651	0	1039	1661	0	526	3223	1455	758	3471	1538
Satd. Flow (RTOR)		81			29				89			89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	4%	5%	3%	17%	0%	12%	11%	9%	4%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	34	218	0	162	165	0	39	467	70	52	793	25
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		55.7	48.2	48.2	57.3	50.9	50.9
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.51	0.44	0.44	0.52	0.46	0.46
v/c Ratio	0.08	0.35		0.45	0.28		0.11	0.33	0.10	0.11	0.49	0.03
Control Delay	25.1	18.3		32.9	22.8		12.6	22.0	3.0	23.0	41.6	8.7
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.1	18.3		32.9	22.8		12.6	22.0	3.0	23.0	41.6	8.7
LOS	C	B		C	C		B	C	A	C	D	A
Approach Delay		19.2			27.8			19.0			39.6	
Approach LOS		B			C			B			D	
Queue Length 50th (m)	5.2	21.9		28.1	21.8		3.9	36.9	0.0	10.9	99.3	0.3
Queue Length 95th (m)	12.6	42.4		49.3	39.1		9.0	51.5	6.2	m19.0	120.1	m2.5
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	412	623		358	592		393	1411	686	485	1605	759

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Existing Volumes 2024
 AM Peak Hour

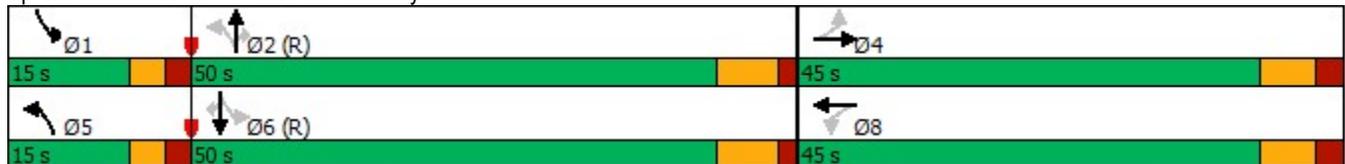


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.35		0.45	0.28		0.10	0.33	0.10	0.11	0.49	0.03

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.49
 Intersection Signal Delay: 29.3
 Intersection LOS: C
 Intersection Capacity Utilization 67.4%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Existing Volumes 2024
 AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	60	38	61	11	30	59	13	281	19	107	747	111
Future Volume (vph)	60	38	61	11	30	59	13	281	19	107	747	111
Satd. Flow (prot)	1583	1712	1495	1421	1776	1442	1570	3167	1380	1656	3505	1495
Flt Permitted	0.736			0.730			0.235			0.566		
Satd. Flow (perm)	1227	1712	1495	1092	1776	1442	388	3167	1380	987	3505	1495
Satd. Flow (RTOR)			92			92			91			121
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	11%	8%	27%	7%	12%	15%	14%	17%	9%	3%	8%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	65	41	66	12	33	64	14	305	21	116	812	121
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effect Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.15	0.07	0.11	0.03	0.05	0.12	0.05	0.25	0.04	0.21	0.60	0.19
Control Delay	26.4	24.8	2.9	24.5	24.6	2.8	6.5	11.0	0.1	13.4	29.0	4.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.4	24.8	2.9	24.5	24.6	2.8	6.5	11.0	0.1	13.4	29.0	4.7
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		17.0			11.8			10.2			24.5	
Approach LOS		B			B			B			C	
Queue Length 50th (m)	10.1	6.2	0.0	1.8	5.0	0.0	0.6	8.0	0.0	12.2	75.7	0.0
Queue Length 95th (m)	20.9	14.2	5.4	6.3	12.0	5.0	m2.0	11.1	0.0	21.6	96.4	11.8
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	421	588	574	375	610	555	301	1229	591	554	1360	654
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.07	0.11	0.03	0.05	0.12	0.05	0.25	0.04	0.21	0.60	0.19

Intersection Summary

Cycle Length: 110	
Actuated Cycle Length: 110	
Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green	
Natural Cycle: 65	
Control Type: Pretimed	
Maximum v/c Ratio: 0.60	
Intersection Signal Delay: 20.0	Intersection LOS: B
Intersection Capacity Utilization 63.7%	ICU Level of Service B
Analysis Period (min) 15	
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis

7: Hwy 62 & Roy Blvd

Existing Volumes 2024
AM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	7	22	68	399	779	23
Future Volume (Veh/h)	7	22	68	399	779	23
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	24	74	434	847	25
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1224	436	872			
vC1, stage 1 conf vol	860					
vC2, stage 2 conf vol	365					
vCu, unblocked vol	1224	436	872			
tC, single (s)	7.1	7.2	4.2			
tC, 2 stage (s)	6.1					
tF (s)	3.6	3.4	2.2			
p0 queue free %	97	96	90			
cM capacity (veh/h)	312	536	763			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	32	74	217	217	565	307
Volume Left	8	74	0	0	0	0
Volume Right	24	0	0	0	0	25
cSH	455	763	1700	1700	1700	1700
Volume to Capacity	0.07	0.10	0.13	0.13	0.33	0.18
Queue Length 95th (m)	1.8	2.6	0.0	0.0	0.0	0.0
Control Delay (s)	13.5	10.2	0.0	0.0	0.0	0.0
Lane LOS	B	B				
Approach Delay (s)	13.5	1.5	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			39.4%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Existing Volumes 2024
 AM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	57	10	86	26	8	135
Future Volume (Veh/h)	57	10	86	26	8	135
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	62	11	93	28	9	147
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	272	107			121	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	272	107			121	
tC, single (s)	6.4	6.2			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.3	
p0 queue free %	91	99			99	
cM capacity (veh/h)	717	953			1407	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	73	121	156			
Volume Left	62	0	9			
Volume Right	11	28	0			
cSH	745	1700	1407			
Volume to Capacity	0.10	0.07	0.01			
Queue Length 95th (m)	2.6	0.0	0.2			
Control Delay (s)	10.4	0.0	0.5			
Lane LOS	B		A			
Approach Delay (s)	10.4	0.0	0.5			
Approach LOS	B					
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utilization		24.1%		ICU Level of Service		A
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

Existing Volumes 2024

1: Maitland Dr & Hwy 62

PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	52	105	61	171	84	65	106	798	113	45	633	40
Future Volume (vph)	52	105	61	171	84	65	106	798	113	45	633	40
Satd. Flow (prot)	1770	1760	0	1736	1681	0	1787	3539	1553	1556	3471	1568
Flt Permitted	0.652			0.627			0.288			0.238		
Satd. Flow (perm)	1215	1760	0	1145	1681	0	542	3539	1553	390	3471	1568
Satd. Flow (RTOR)		29			39				106			89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	4%	6%	5%	1%	2%	4%	16%	4%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	57	180	0	186	162	0	115	867	123	49	688	43
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		57.2	48.1	48.1	53.8	44.6	44.6
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.52	0.44	0.44	0.49	0.41	0.41
v/c Ratio	0.14	0.29		0.47	0.27		0.30	0.56	0.17	0.18	0.49	0.06
Control Delay	25.9	23.2		33.0	20.9		14.4	25.6	5.9	22.2	40.8	11.2
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.9	23.2		33.0	20.9		14.4	25.6	5.9	22.2	40.8	11.2
LOS	C	C		C	C		B	C	A	C	D	B
Approach Delay		23.9			27.3			22.2			38.0	
Approach LOS		C			C			C			D	
Queue Length 50th (m)	8.8	24.2		32.6	19.5		12.0	77.7	2.2	8.5	79.1	1.4
Queue Length 95th (m)	18.6	42.3		55.2	36.6		21.3	101.3	13.9	18.5	99.1	8.7
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	419	626		395	606		398	1548	739	305	1406	688

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Existing Volumes 2024
 PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.29		0.47	0.27		0.29	0.56	0.17	0.16	0.49	0.06

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.56
 Intersection Signal Delay: 28.1
 Intersection LOS: C
 Intersection Capacity Utilization 68.0%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Existing Volumes 2024
 PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	137	32	48	28	48	149	47	739	21	46	451	68
Future Volume (vph)	137	32	48	28	48	149	47	739	21	46	451	68
Satd. Flow (prot)	1787	1845	1524	1570	1900	1568	1687	3539	1346	1687	3406	1404
Flt Permitted	0.723			0.734			0.429			0.239		
Satd. Flow (perm)	1360	1845	1524	1213	1900	1568	762	3539	1346	424	3406	1404
Satd. Flow (RTOR)			92			162			91			91
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	6%	15%	0%	3%	7%	2%	20%	7%	6%	15%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	149	35	52	30	52	162	51	803	23	50	490	74
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effect Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.32	0.06	0.09	0.07	0.08	0.25	0.11	0.58	0.04	0.15	0.37	0.12
Control Delay	29.0	24.6	1.5	25.1	24.9	5.0	5.5	12.3	0.1	13.0	25.1	3.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.0	24.6	1.5	25.1	24.9	5.0	5.5	12.3	0.1	13.0	25.1	3.6
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		22.3			11.7			11.6			21.5	
Approach LOS		C			B			B			C	
Queue Length 50th (m)	24.6	5.2	0.0	4.5	7.9	0.0	1.9	17.8	0.0	5.1	41.0	0.0
Queue Length 95th (m)	42.5	12.6	2.4	11.6	16.8	14.4	m3.4	21.7	m0.0	11.0	55.2	6.9
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	467	634	584	416	652	645	465	1373	578	326	1322	600
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.06	0.09	0.07	0.08	0.25	0.11	0.58	0.04	0.15	0.37	0.12

Intersection Summary

Cycle Length: 110	
Actuated Cycle Length: 110	
Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green	
Natural Cycle: 65	
Control Type: Pretimed	
Maximum v/c Ratio: 0.58	
Intersection Signal Delay: 16.0	Intersection LOS: B
Intersection Capacity Utilization 63.5%	ICU Level of Service B
Analysis Period (min) 15	
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Existing Volumes 2024
PM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	30	96	68	844	610	19
Future Volume (Veh/h)	30	96	68	844	610	19
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	33	104	74	917	663	21
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1280	342	684			
vC1, stage 1 conf vol	674					
vC2, stage 2 conf vol	606					
vCu, unblocked vol	1280	342	684			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	91	84	92			
cM capacity (veh/h)	355	657	905			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	137	74	458	458	442	242
Volume Left	33	74	0	0	0	0
Volume Right	104	0	0	0	0	21
cSH	545	905	1700	1700	1700	1700
Volume to Capacity	0.25	0.08	0.27	0.27	0.26	0.14
Queue Length 95th (m)	7.9	2.1	0.0	0.0	0.0	0.0
Control Delay (s)	13.8	9.3	0.0	0.0	0.0	0.0
Lane LOS	B	A				
Approach Delay (s)	13.8	0.7	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utilization			38.8%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Existing Volumes 2024
 PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	26	5	254	50	13	111
Future Volume (Veh/h)	26	5	254	50	13	111
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	28	5	276	54	14	121
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	452	303			330	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	452	303			330	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	95	99			99	
cM capacity (veh/h)	563	741			1241	
Direction, Lane #						
	WB 1	NB 1	SB 1			
Volume Total	33	330	135			
Volume Left	28	0	14			
Volume Right	5	54	0			
cSH	584	1700	1241			
Volume to Capacity	0.06	0.19	0.01			
Queue Length 95th (m)	1.4	0.0	0.3			
Control Delay (s)	11.5	0.0	0.9			
Lane LOS	B		A			
Approach Delay (s)	11.5	0.0	0.9			
Approach LOS	B					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization			26.7%	ICU Level of Service	A	
Analysis Period (min)			15			

Appendix F

Synchro Reports

Background Volumes 2027

HCM Signalized Intersection Capacity Analysis

Background Volumes 2027

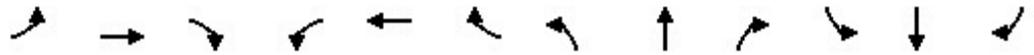
1: Maitland Dr & Hwy 62

AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	32	80	129	156	101	57	38	449	67	51	764	24
Future Volume (vph)	32	80	129	156	101	57	38	449	67	51	764	24
Satd. Flow (prot)	1752	1651	0	1719	1664	0	1805	3223	1455	1656	3471	1538
Flt Permitted	0.638			0.562			0.259			0.420		
Satd. Flow (perm)	1177	1651	0	1017	1664	0	492	3223	1455	732	3471	1538
Satd. Flow (RTOR)		80			28				89			89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	4%	5%	3%	17%	0%	12%	11%	9%	4%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	35	227	0	170	172	0	41	488	73	55	830	26
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		55.6	48.1	48.1	57.4	50.9	50.9
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.51	0.44	0.44	0.52	0.46	0.46
v/c Ratio	0.09	0.36		0.48	0.29		0.12	0.35	0.11	0.12	0.52	0.03
Control Delay	25.2	19.0		34.0	23.3		12.7	22.3	3.4	22.7	41.9	8.6
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.2	19.0		34.0	23.3		12.7	22.3	3.4	22.7	41.9	8.6
LOS	C	B		C	C		B	C	A	C	D	A
Approach Delay		19.8			28.6			19.3			39.8	
Approach LOS		B			C			B			D	
Queue Length 50th (m)	5.3	23.8		30.0	23.1		4.1	38.9	0.0	11.6	104.0	0.3
Queue Length 95th (m)	12.9	44.9		52.3	41.1		9.4	54.3	6.7	m19.5	125.0	m2.4
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	406	622		351	593		377	1408	685	473	1604	758

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Background Volumes 2027
 AM Peak Hour

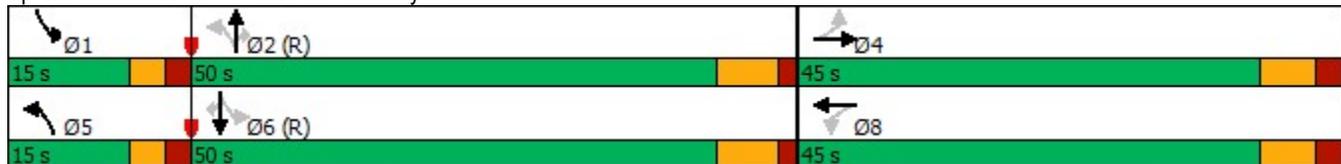


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.36		0.48	0.29		0.11	0.35	0.11	0.12	0.52	0.03

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.52
 Intersection Signal Delay: 29.7
 Intersection LOS: C
 Intersection Capacity Utilization 69.1%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
9: Hwy 62 & Vermilyea Rd/Farnham Rd

Background Volumes 2027
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	62	40	64	12	31	61	14	294	19	112	781	116
Future Volume (vph)	62	40	64	12	31	61	14	294	19	112	781	116
Satd. Flow (prot)	1583	1712	1495	1421	1776	1442	1570	3167	1380	1656	3505	1495
Flt Permitted	0.735			0.729			0.217			0.558		
Satd. Flow (perm)	1225	1712	1495	1091	1776	1442	359	3167	1380	973	3505	1495
Satd. Flow (RTOR)			92			92			91			126
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	11%	8%	27%	7%	12%	15%	14%	17%	9%	3%	8%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	67	43	70	13	34	66	15	320	21	122	849	126
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effect Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.16	0.07	0.12	0.03	0.06	0.12	0.05	0.26	0.04	0.22	0.62	0.19
Control Delay	26.4	24.9	3.4	24.5	24.6	2.9	6.6	11.0	0.1	13.5	29.7	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.4	24.9	3.4	24.5	24.6	2.9	6.6	11.0	0.1	13.5	29.7	4.6
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		17.1			11.9			10.2			25.0	
Approach LOS		B			B			B			C	
Queue Length 50th (m)	10.4	6.5	0.0	2.0	5.1	0.0	0.7	8.2	0.0	12.9	80.3	0.0
Queue Length 95th (m)	21.4	14.8	6.4	6.4	12.3	5.4	m1.9	11.5	0.0	22.6	101.8	12.0
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	420	588	574	374	610	555	289	1229	591	548	1360	657
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.07	0.12	0.03	0.06	0.12	0.05	0.26	0.04	0.22	0.62	0.19

Intersection Summary

Cycle Length: 110	
Actuated Cycle Length: 110	
Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green	
Natural Cycle: 65	
Control Type: Pretimed	
Maximum v/c Ratio: 0.62	
Intersection Signal Delay: 20.3	Intersection LOS: C
Intersection Capacity Utilization 64.7%	ICU Level of Service C
Analysis Period (min) 15	
m Volume for 95th percentile queue is metered by upstream signal.	

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Background Volumes 2027
AM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	8	23	71	417	814	24
Future Volume (Veh/h)	8	23	71	417	814	24
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	25	77	453	885	26
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1278	456	911			
vC1, stage 1 conf vol	898					
vC2, stage 2 conf vol	380					
vCu, unblocked vol	1278	456	911			
tC, single (s)	7.1	7.2	4.2			
tC, 2 stage (s)	6.1					
tF (s)	3.6	3.4	2.2			
p0 queue free %	97	95	90			
cM capacity (veh/h)	297	520	737			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	34	77	226	226	590	321
Volume Left	9	77	0	0	0	0
Volume Right	25	0	0	0	0	26
cSH	434	737	1700	1700	1700	1700
Volume to Capacity	0.08	0.10	0.13	0.13	0.35	0.19
Queue Length 95th (m)	2.0	2.8	0.0	0.0	0.0	0.0
Control Delay (s)	14.0	10.5	0.0	0.0	0.0	0.0
Lane LOS	B	B				
Approach Delay (s)	14.0	1.5	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			40.5%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Background Volumes 2027
 AM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	83	15	87	34	10	137
Future Volume (Veh/h)	83	15	87	34	10	137
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	90	16	95	37	11	149
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	284	114			132	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	284	114			132	
tC, single (s)	6.4	6.2			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.3	
p0 queue free %	87	98			99	
cM capacity (veh/h)	704	945			1394	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	106	132	160			
Volume Left	90	0	11			
Volume Right	16	37	0			
cSH	733	1700	1394			
Volume to Capacity	0.14	0.08	0.01			
Queue Length 95th (m)	4.0	0.0	0.2			
Control Delay (s)	10.7	0.0	0.6			
Lane LOS	B		A			
Approach Delay (s)	10.7	0.0	0.6			
Approach LOS	B					
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			27.6%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

Background Volumes 2027

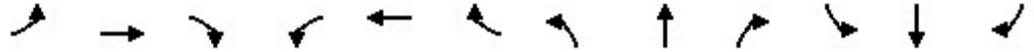
1: Maitland Dr & Hwy 62

PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	54	110	64	179	88	68	111	835	119	47	661	42
Future Volume (vph)	54	110	64	179	88	68	111	835	119	47	661	42
Satd. Flow (prot)	1770	1760	0	1736	1683	0	1787	3539	1553	1556	3471	1568
Flt Permitted	0.641			0.613			0.272			0.219		
Satd. Flow (perm)	1194	1760	0	1120	1683	0	512	3539	1553	359	3471	1568
Satd. Flow (RTOR)		29			39				106			89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	4%	6%	5%	1%	2%	4%	16%	4%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	59	190	0	195	170	0	121	908	129	51	718	46
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		57.2	48.1	48.1	53.8	44.5	44.5
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.52	0.44	0.44	0.49	0.40	0.40
v/c Ratio	0.14	0.30		0.51	0.28		0.33	0.59	0.17	0.20	0.51	0.07
Control Delay	26.1	23.7		34.1	21.3		14.8	26.2	6.4	22.4	41.3	11.9
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.1	23.7		34.1	21.3		14.8	26.2	6.4	22.4	41.3	11.9
LOS	C	C		C	C		B	C	A	C	D	B
Approach Delay		24.3			28.2			22.8			38.5	
Approach LOS		C			C			C			D	
Queue Length 50th (m)	9.1	26.0		34.7	20.8		12.7	82.6	3.0	8.8	82.8	1.6
Queue Length 95th (m)	19.2	44.9		58.3	38.6		22.4	107.4	15.1	18.9	103.2	9.3
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	412	626		386	606		384	1547	738	292	1403	687

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Background Volumes 2027
 PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.14	0.30		0.51	0.28		0.32	0.59	0.17	0.17	0.51	0.07

Intersection Summary

Cycle Length: 110	
Actuated Cycle Length: 110	
Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green	
Natural Cycle: 75	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.59	
Intersection Signal Delay: 28.6	Intersection LOS: C
Intersection Capacity Utilization 69.9%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
9: Hwy 62 & Vermilyea Rd/Farnham Rd

Background Volumes 2027
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	143	33	51	29	51	156	50	772	22	48	472	71
Future Volume (vph)	143	33	51	29	51	156	50	772	22	48	472	71
Satd. Flow (prot)	1787	1845	1524	1570	1900	1568	1687	3539	1346	1687	3406	1404
Flt Permitted	0.721			0.734			0.413			0.221		
Satd. Flow (perm)	1356	1845	1524	1213	1900	1568	733	3539	1346	392	3406	1404
Satd. Flow (RTOR)			92			170			91			91
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	6%	15%	0%	3%	7%	2%	20%	7%	6%	15%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	155	36	55	32	55	170	54	839	24	52	513	77
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effect Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.33	0.06	0.09	0.08	0.08	0.26	0.12	0.61	0.04	0.17	0.39	0.13
Control Delay	29.3	24.6	1.8	25.1	25.0	5.0	5.5	12.2	0.1	13.2	25.3	3.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.3	24.6	1.8	25.1	25.0	5.0	5.5	12.2	0.1	13.2	25.3	3.9
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		22.5			11.8			11.5			21.8	
Approach LOS		C			B			B			C	
Queue Length 50th (m)	25.8	5.4	0.0	4.8	8.3	0.0	1.9	18.6	0.0	5.3	43.3	0.0
Queue Length 95th (m)	44.0	12.8	3.2	12.1	17.7	14.7	m3.4	22.5	m0.0	11.3	57.9	7.5
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	465	634	584	416	652	650	453	1373	578	313	1322	600
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.06	0.09	0.08	0.08	0.26	0.12	0.61	0.04	0.17	0.39	0.13

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.61
 Intersection Signal Delay: 16.0 Intersection LOS: B
 Intersection Capacity Utilization 64.4% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Background Volumes 2027
PM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	31	100	71	882	638	19
Future Volume (Veh/h)	31	100	71	882	638	19
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	109	77	959	693	21
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1337	357	714			
vC1, stage 1 conf vol	704					
vC2, stage 2 conf vol	634					
vCu, unblocked vol	1337	357	714			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	90	83	91			
cM capacity (veh/h)	340	642	882			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	143	77	480	480	462	252
Volume Left	34	77	0	0	0	0
Volume Right	109	0	0	0	0	21
cSH	530	882	1700	1700	1700	1700
Volume to Capacity	0.27	0.09	0.28	0.28	0.27	0.15
Queue Length 95th (m)	8.7	2.3	0.0	0.0	0.0	0.0
Control Delay (s)	14.3	9.5	0.0	0.0	0.0	0.0
Lane LOS	B	A				
Approach Delay (s)	14.3	0.7	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization			40.1%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Background Volumes 2027
 PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	43	8	258	77	21	113
Future Volume (Veh/h)	43	8	258	77	21	113
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	47	9	280	84	23	123
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	491	322			364	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	491	322			364	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	91	99			98	
cM capacity (veh/h)	530	724			1206	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	56	364	146			
Volume Left	47	0	23			
Volume Right	9	84	0			
cSH	554	1700	1206			
Volume to Capacity	0.10	0.21	0.02			
Queue Length 95th (m)	2.7	0.0	0.5			
Control Delay (s)	12.2	0.0	1.4			
Lane LOS	B		A			
Approach Delay (s)	12.2	0.0	1.4			
Approach LOS	B					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			33.8%		ICU Level of Service	A
Analysis Period (min)			15			

Appendix G

Synchro Reports

Background Volumes 2032

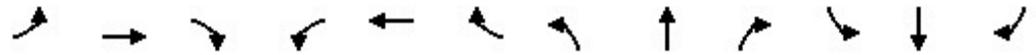
HCM Signalized Intersection Capacity Analysis
1: Maitland Dr & Hwy 62

Background Volumes 2032
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	35	86	139	168	109	62	41	484	72	55	823	26
Future Volume (vph)	35	86	139	168	109	62	41	484	72	55	823	26
Satd. Flow (prot)	1752	1651	0	1719	1663	0	1805	3223	1455	1656	3471	1538
Flt Permitted	0.620			0.539			0.216			0.407		
Satd. Flow (perm)	1144	1651	0	975	1663	0	410	3223	1455	709	3471	1538
Satd. Flow (RTOR)		81			28				89			89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	4%	5%	3%	17%	0%	12%	11%	9%	4%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	38	244	0	183	185	0	45	526	78	60	895	28
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		55.6	48.0	48.0	56.4	48.4	48.4
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.51	0.44	0.44	0.51	0.44	0.44
v/c Ratio	0.10	0.39		0.54	0.31		0.15	0.37	0.11	0.14	0.59	0.04
Control Delay	25.4	19.9		36.4	24.0		13.0	22.8	4.0	22.2	44.7	9.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.4	19.9		36.4	24.0		13.0	22.8	4.0	22.2	44.7	9.0
LOS	C	B		D	C		B	C	A	C	D	A
Approach Delay		20.7			30.1			19.8			42.3	
Approach LOS		C			C			B			D	
Queue Length 50th (m)	5.8	26.8		33.1	25.5		4.5	42.5	0.0	12.7	112.3	0.4
Queue Length 95th (m)	13.6	49.2		57.3	44.4		10.0	59.2	7.8	m20.1	134.1	m2.2
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	395	623		336	592		341	1405	684	460	1527	726

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Background Volumes 2032
 AM Peak Hour

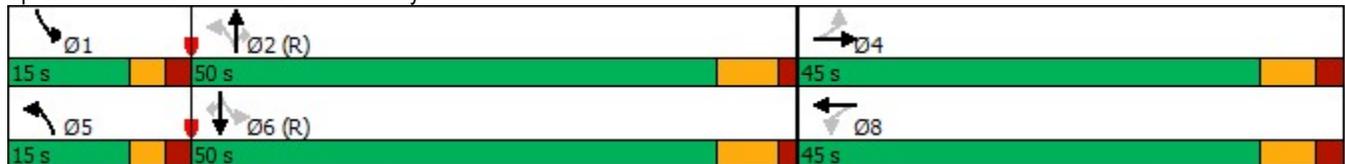


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.39		0.54	0.31		0.13	0.37	0.11	0.13	0.59	0.04

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.59
 Intersection Signal Delay: 31.3
 Intersection LOS: C
 Intersection Capacity Utilization 72.4%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Background Volumes 2032
 AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	67	43	68	13	34	66	15	317	21	121	841	125
Future Volume (vph)	67	43	68	13	34	66	15	317	21	121	841	125
Satd. Flow (prot)	1583	1712	1495	1421	1776	1442	1570	3167	1380	1656	3505	1495
Flt Permitted	0.733			0.726			0.186			0.541		
Satd. Flow (perm)	1222	1712	1495	1086	1776	1442	307	3167	1380	943	3505	1495
Satd. Flow (RTOR)			92			92			91			129
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	11%	8%	27%	7%	12%	15%	14%	17%	9%	3%	8%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	73	47	74	14	37	72	16	345	23	132	914	136
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effect Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.17	0.08	0.13	0.04	0.06	0.13	0.06	0.28	0.04	0.25	0.67	0.21
Control Delay	26.7	25.0	4.0	24.5	24.7	3.7	6.5	11.1	0.1	13.8	30.9	5.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.7	25.0	4.0	24.5	24.7	3.7	6.5	11.1	0.1	13.8	30.9	5.3
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		17.6			12.4			10.3			26.0	
Approach LOS		B			B			B			C	
Queue Length 50th (m)	11.4	7.1	0.0	2.1	5.6	0.0	0.7	8.8	0.0	14.0	88.6	1.0
Queue Length 95th (m)	22.9	15.8	7.3	6.8	13.1	6.8	m2.0	12.2	0.0	24.3	111.8	13.4
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	419	588	574	373	610	555	268	1229	591	536	1360	659
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.08	0.13	0.04	0.06	0.13	0.06	0.28	0.04	0.25	0.67	0.21

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.67
 Intersection Signal Delay: 21.0 Intersection LOS: C
 Intersection Capacity Utilization 66.3% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Background Volumes 2032
AM Peak Hour



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	8	24	77	449	877	26
Future Volume (Veh/h)	8	24	77	449	877	26
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	26	84	488	953	28
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1379	490	981			
vC1, stage 1 conf vol	967					
vC2, stage 2 conf vol	412					
vCu, unblocked vol	1379	490	981			
tC, single (s)	7.1	7.2	4.2			
tC, 2 stage (s)	6.1					
tF (s)	3.6	3.4	2.2			
p0 queue free %	97	95	88			
cM capacity (veh/h)	271	493	693			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	35	84	244	244	635	346
Volume Left	9	84	0	0	0	0
Volume Right	26	0	0	0	0	28
cSH	407	693	1700	1700	1700	1700
Volume to Capacity	0.09	0.12	0.14	0.14	0.37	0.20
Queue Length 95th (m)	2.2	3.3	0.0	0.0	0.0	0.0
Control Delay (s)	14.7	10.9	0.0	0.0	0.0	0.0
Lane LOS	B	B				
Approach Delay (s)	14.7	1.6	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			42.7%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
11: Sidney St & Kempton Ave

Background Volumes 2032
AM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	87	16	93	36	11	148
Future Volume (Veh/h)	87	16	93	36	11	148
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	95	17	101	39	12	161
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	306	120			140	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	306	120			140	
tC, single (s)	6.4	6.2			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.3	
p0 queue free %	86	98			99	
cM capacity (veh/h)	685	936			1384	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	112	140	173			
Volume Left	95	0	12			
Volume Right	17	39	0			
cSH	714	1700	1384			
Volume to Capacity	0.16	0.08	0.01			
Queue Length 95th (m)	4.4	0.0	0.2			
Control Delay (s)	11.0	0.0	0.6			
Lane LOS	B		A			
Approach Delay (s)	11.0	0.0	0.6			
Approach LOS	B					
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			29.3%		ICU Level of Service	A
Analysis Period (min)			15			

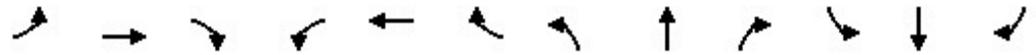
HCM Signalized Intersection Capacity Analysis
1: Maitland Dr & Hwy 62

Background Volumes 2032
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	58	118	68	193	95	73	120	899	128	51	713	45
Future Volume (vph)	58	118	68	193	95	73	120	899	128	51	713	45
Satd. Flow (prot)	1770	1760	0	1736	1683	0	1787	3539	1553	1556	3471	1568
Flt Permitted	0.624			0.596			0.243			0.188		
Satd. Flow (perm)	1162	1760	0	1089	1683	0	457	3539	1553	308	3471	1568
Satd. Flow (RTOR)		29			38				106			89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	4%	6%	5%	1%	2%	4%	16%	4%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	63	202	0	210	182	0	130	977	139	55	775	49
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		57.2	48.0	48.0	53.8	44.4	44.4
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.52	0.44	0.44	0.49	0.40	0.40
v/c Ratio	0.16	0.32		0.56	0.30		0.38	0.63	0.19	0.23	0.55	0.07
Control Delay	26.3	24.2		36.1	22.2		15.7	27.3	7.1	23.0	42.2	12.2
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.3	24.2		36.1	22.2		15.7	27.3	7.1	23.0	42.2	12.2
LOS	C	C		D	C		B	C	A	C	D	B
Approach Delay		24.7			29.6			23.8			39.3	
Approach LOS		C			C			C			D	
Queue Length 50th (m)	9.8	28.2		38.2	23.2		13.7	91.5	4.3	9.6	90.2	1.9
Queue Length 95th (m)	20.4	47.9		63.8	41.5		23.7	118.4	17.2	20.0	111.2	10.4
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	401	626		376	606		360	1543	737	270	1399	685

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Background Volumes 2032
 PM Peak Hour

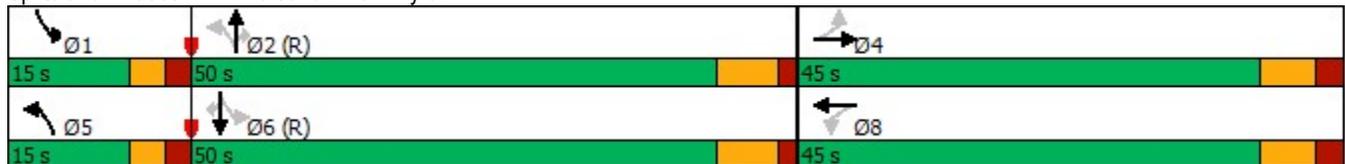


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.32		0.56	0.30		0.36	0.63	0.19	0.20	0.55	0.07

Intersection Summary

Cycle Length: 110	
Actuated Cycle Length: 110	
Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green	
Natural Cycle: 75	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.63	
Intersection Signal Delay: 29.6	Intersection LOS: C
Intersection Capacity Utilization 73.2%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
9: Hwy 62 & Vermilyea Rd/Farnham Rd

Background Volumes 2032
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	154	36	55	31	55	168	53	832	23	52	508	77
Future Volume (vph)	154	36	55	31	55	168	53	832	23	52	508	77
Satd. Flow (prot)	1787	1845	1524	1570	1900	1568	1687	3539	1346	1687	3406	1404
Flt Permitted	0.718			0.732			0.387			0.191		
Satd. Flow (perm)	1351	1845	1524	1209	1900	1568	687	3539	1346	339	3406	1404
Satd. Flow (RTOR)			92			183			91			91
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	6%	15%	0%	3%	7%	2%	20%	7%	6%	15%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	167	39	60	34	60	183	58	904	25	57	552	84
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effect Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.36	0.06	0.10	0.08	0.09	0.28	0.13	0.66	0.04	0.20	0.42	0.14
Control Delay	29.8	24.7	2.4	25.2	25.1	4.9	5.5	12.7	0.1	13.6	25.8	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.8	24.7	2.4	25.2	25.1	4.9	5.5	12.7	0.1	13.6	25.8	4.6
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		22.9			11.8			12.0			22.2	
Approach LOS		C			B			B			C	
Queue Length 50th (m)	28.1	5.9	0.0	5.2	9.1	0.0	2.0	20.1	0.0	5.8	47.3	0.0
Queue Length 95th (m)	47.4	13.6	4.2	12.6	18.8	15.1	m3.4	24.0	m0.0	12.1	62.7	8.9
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	464	634	584	415	652	658	434	1373	578	292	1322	600
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.36	0.06	0.10	0.08	0.09	0.28	0.13	0.66	0.04	0.20	0.42	0.14

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.66
 Intersection Signal Delay: 16.5
 Intersection LOS: B
 Intersection Capacity Utilization 66.1%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Background Volumes 2032
PM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	34	108	77	950	687	21
Future Volume (Veh/h)	34	108	77	950	687	21
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	37	117	84	1033	747	23
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1443	385	770			
vC1, stage 1 conf vol	758					
vC2, stage 2 conf vol	684					
vCu, unblocked vol	1443	385	770			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	88	81	90			
cM capacity (veh/h)	313	616	840			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	154	84	516	516	498	272
Volume Left	37	84	0	0	0	0
Volume Right	117	0	0	0	0	23
cSH	500	840	1700	1700	1700	1700
Volume to Capacity	0.31	0.10	0.30	0.30	0.29	0.16
Queue Length 95th (m)	10.4	2.7	0.0	0.0	0.0	0.0
Control Delay (s)	15.4	9.8	0.0	0.0	0.0	0.0
Lane LOS	C	A				
Approach Delay (s)	15.4	0.7	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			42.5%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Background Volumes 2032
 PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	45	9	278	81	22	122
Future Volume (Veh/h)	45	9	278	81	22	122
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	10	302	88	24	133
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	527	346			390	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	527	346			390	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	90	99			98	
cM capacity (veh/h)	505	702			1180	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	59	390	157			
Volume Left	49	0	24			
Volume Right	10	88	0			
cSH	530	1700	1180			
Volume to Capacity	0.11	0.23	0.02			
Queue Length 95th (m)	3.0	0.0	0.5			
Control Delay (s)	12.6	0.0	1.4			
Lane LOS	B		A			
Approach Delay (s)	12.6	0.0	1.4			
Approach LOS	B					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			35.2%		ICU Level of Service	A
Analysis Period (min)			15			

Appendix H

Synchro Reports

Total Volumes 2027

HCM Signalized Intersection Capacity Analysis

Total Volumes 2027

1: Maitland Dr & Hwy 62

AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	33	80	129	156	101	58	38	457	67	55	816	26
Future Volume (vph)	33	80	129	156	101	58	38	457	67	55	816	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0		0.0	60.0		0.0	45.0		50.0	45.0		50.0
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (m)	100.0			65.0			0.0			0.0		
Satd. Flow (prot)	1752	1651	0	1719	1661	0	1805	3223	1455	1656	3471	1538
Flt Permitted	0.637			0.562			0.233			0.413		
Satd. Flow (perm)	1175	1651	0	1017	1661	0	443	3223	1455	720	3471	1538
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		80			29				89			89
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		927.0			367.9			238.3			786.0	
Travel Time (s)		66.7			26.5			14.3			47.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	4%	5%	3%	17%	0%	12%	11%	9%	4%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	36	227	0	170	173	0	41	497	73	60	887	28
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		55.5	48.0	48.0	57.5	50.9	50.9
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.50	0.44	0.44	0.52	0.46	0.46
v/c Ratio	0.09	0.36		0.48	0.29		0.13	0.35	0.11	0.14	0.55	0.04
Control Delay	25.2	19.0		34.0	23.2		12.8	22.5	3.5	22.0	41.5	8.3
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.2	19.0		34.0	23.2		12.8	22.5	3.5	22.0	41.5	8.3

HCM Signalized Intersection Capacity Analysis
 1: Maitland Dr & Hwy 62

Total Volumes 2027
 AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	C	B		C	C		B	C	A	C	D	A
Approach Delay		19.8			28.6			19.5			39.3	
Approach LOS		B			C			B			D	
Queue Length 50th (m)	5.4	23.8		30.0	23.2		4.1	39.8	0.0	12.3	109.7	0.4
Queue Length 95th (m)	13.1	44.9		52.3	41.1		9.4	55.6	6.8	m20.3	131.4	m2.5
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	405	622		351	592		356	1405	684	468	1604	758
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.36		0.48	0.29		0.12	0.35	0.11	0.13	0.55	0.04

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.55
 Intersection Signal Delay: 29.8 Intersection LOS: C
 Intersection Capacity Utilization 70.6% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Total Volumes 2027
 AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	62	40	65	12	31	61	15	314	21	112	796	116
Future Volume (vph)	62	40	65	12	31	61	15	314	21	112	796	116
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	80.0			100.0			7.5			100.0		
Satd. Flow (prot)	1583	1712	1495	1421	1776	1442	1570	3167	1380	1656	3505	1495
Flt Permitted	0.735			0.729			0.209			0.545		
Satd. Flow (perm)	1225	1712	1495	1091	1776	1442	345	3167	1380	950	3505	1495
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			92			91			126
Link Speed (k/h)		50			50			80				50
Link Distance (m)		249.3			317.7			872.4				250.2
Travel Time (s)		17.9			22.9			39.3				18.0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	11%	8%	27%	7%	12%	15%	14%	17%	9%	3%	8%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	67	43	71	13	34	66	16	341	23	122	865	126
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6				3.6
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		4.8			4.8			4.8				4.8
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effct Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.16	0.07	0.12	0.03	0.06	0.12	0.06	0.28	0.04	0.23	0.64	0.19
Control Delay	26.4	24.9	3.5	24.5	24.6	2.9	6.8	11.7	0.1	13.6	29.9	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.4	24.9	3.5	24.5	24.6	2.9	6.8	11.7	0.1	13.6	29.9	4.6
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		17.1			11.9			10.8			25.3	
Approach LOS		B			B			B			C	
Queue Length 50th (m)	10.4	6.5	0.0	2.0	5.1	0.0	0.7	9.7	0.0	12.9	82.4	0.0

HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Total Volumes 2027
 AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (m)	21.4	14.8	6.5	6.4	12.3	5.4	m2.2	13.3	0.0	22.6	104.3	12.0
Internal Link Dist (m)	225.3				293.7		848.4				226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	420	588	574	374	610	555	283	1229	591	539	1360	657
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.07	0.12	0.03	0.06	0.12	0.06	0.28	0.04	0.23	0.64	0.19

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.64
 Intersection Signal Delay: 20.5 Intersection LOS: C
 Intersection Capacity Utilization 65.1% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Total Volumes 2027
AM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	31	81	81	417	814	40
Future Volume (Veh/h)	31	81	81	417	814	40
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	88	88	453	885	43
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1309	464	928			
vC1, stage 1 conf vol	906					
vC2, stage 2 conf vol	402					
vCu, unblocked vol	1309	464	928			
tC, single (s)	7.1	7.2	4.2			
tC, 2 stage (s)	6.1					
tF (s)	3.6	3.4	2.2			
p0 queue free %	88	83	88			
cM capacity (veh/h)	290	514	726			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	122	88	226	226	590	338
Volume Left	34	88	0	0	0	0
Volume Right	88	0	0	0	0	43
cSH	423	726	1700	1700	1700	1700
Volume to Capacity	0.29	0.12	0.13	0.13	0.35	0.20
Queue Length 95th (m)	9.4	3.3	0.0	0.0	0.0	0.0
Control Delay (s)	16.9	10.6	0.0	0.0	0.0	0.0
Lane LOS	C	B				
Approach Delay (s)	16.9	1.7	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utilization			45.0%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Total Volumes 2027
 AM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	86	16	87	35	10	137
Future Volume (Veh/h)	86	16	87	35	10	137
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	93	17	95	38	11	149
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	285	114			133	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	285	114			133	
tC, single (s)	6.4	6.2			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.3	
p0 queue free %	87	98			99	
cM capacity (veh/h)	704	944			1392	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	110	133	160			
Volume Left	93	0	11			
Volume Right	17	38	0			
cSH	733	1700	1392			
Volume to Capacity	0.15	0.08	0.01			
Queue Length 95th (m)	4.2	0.0	0.2			
Control Delay (s)	10.8	0.0	0.6			
Lane LOS	B		A			
Approach Delay (s)	10.8	0.0	0.6			
Approach LOS	B					
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utilization			27.8%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

Total Volumes 2027

1: Maitland Dr & Hwy 62

PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	57	110	64	179	88	72	111	881	119	49	680	43
Future Volume (vph)	57	110	64	179	88	72	111	881	119	49	680	43
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0		0.0	60.0		0.0	45.0		50.0	45.0		50.0
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (m)	100.0			65.0			0.0			0.0		
Satd. Flow (prot)	1770	1760	0	1736	1679	0	1787	3539	1553	1556	3471	1568
Flt Permitted	0.635			0.613			0.262			0.196		
Satd. Flow (perm)	1183	1760	0	1120	1679	0	493	3539	1553	321	3471	1568
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29			41				100			89
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		927.0			367.9			238.3			786.0	
Travel Time (s)		66.7			26.5			14.3			47.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	4%	6%	5%	1%	2%	4%	16%	4%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	62	190	0	195	174	0	121	958	129	53	739	47
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		57.2	48.0	48.0	53.8	44.5	44.5
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.52	0.44	0.44	0.49	0.40	0.40
v/c Ratio	0.15	0.30		0.51	0.29		0.34	0.62	0.18	0.22	0.53	0.07
Control Delay	26.2	23.7		34.1	21.3		15.0	27.0	7.0	22.6	41.2	11.6
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.2	23.7		34.1	21.3		15.0	27.0	7.0	22.6	41.2	11.6

HCM Signalized Intersection Capacity Analysis

1: Maitland Dr & Hwy 62

Total Volumes 2027

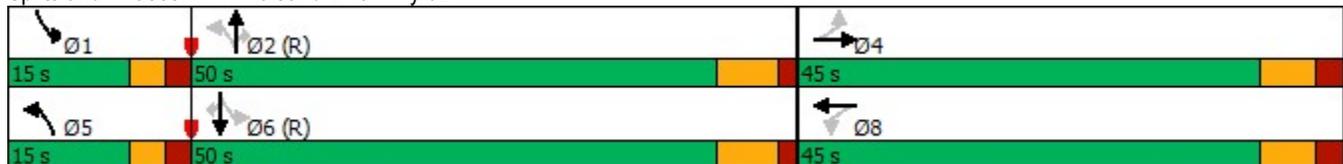
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	C	C		C	C		B	C	A	C	D	B
Approach Delay		24.3			28.1			23.6			38.4	
Approach LOS		C			C			C			D	
Queue Length 50th (m)	9.6	26.0		34.7	21.2		12.7	89.0	3.8	9.1	85.2	1.7
Queue Length 95th (m)	19.9	44.9		58.3	39.0		22.4	115.2	16.1	19.5	106.1	9.6
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	408	626		386	606		376	1545	734	276	1403	687
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.30		0.51	0.29		0.32	0.62	0.18	0.19	0.53	0.07

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.62
Intersection Signal Delay:	28.9
Intersection LOS:	C
Intersection Capacity Utilization	71.2%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Total Volumes 2027
 PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	143	33	54	31	51	156	52	794	23	48	497	71
Future Volume (vph)	143	33	54	31	51	156	52	794	23	48	497	71
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	80.0			100.0			7.5			100.0		
Satd. Flow (prot)	1787	1845	1524	1570	1900	1568	1687	3539	1346	1687	3406	1404
Flt Permitted	0.721			0.734			0.395			0.210		
Satd. Flow (perm)	1356	1845	1524	1213	1900	1568	701	3539	1346	373	3406	1404
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			170			91			91
Link Speed (k/h)		50			50			80				50
Link Distance (m)		249.3			317.7			872.4				250.2
Travel Time (s)		17.9			22.9			39.3				18.0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	6%	15%	0%	3%	7%	2%	20%	7%	6%	15%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	155	36	59	34	55	170	57	863	25	52	540	77
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6				3.6
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		4.8			4.8			4.8				4.8
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effct Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.33	0.06	0.10	0.08	0.08	0.26	0.13	0.63	0.04	0.17	0.41	0.13
Control Delay	29.3	24.6	2.2	25.2	25.0	5.0	5.7	12.7	0.1	13.2	25.6	3.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.3	24.6	2.2	25.2	25.0	5.0	5.7	12.7	0.1	13.2	25.6	3.9
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		22.2			11.9			12.0			22.2	
Approach LOS		C			B			B			C	
Queue Length 50th (m)	25.8	5.4	0.0	5.2	8.3	0.0	2.1	20.4	0.0	5.3	46.0	0.0

HCM Signalized Intersection Capacity Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

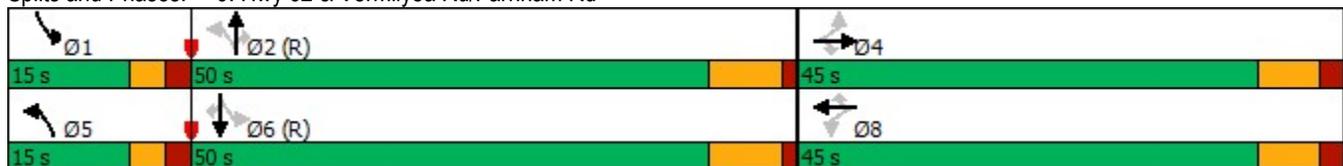
Total Volumes 2027
 PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (m)	44.0	12.8	3.9	12.6	17.7	14.7	m3.6	24.5	m0.0	11.3	61.2	7.5
Internal Link Dist (m)	225.3				293.7		848.4				226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	465	634	584	416	652	650	440	1373	578	305	1322	600
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.06	0.10	0.08	0.08	0.26	0.13	0.63	0.04	0.17	0.41	0.13

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.63
 Intersection Signal Delay: 16.4 Intersection LOS: B
 Intersection Capacity Utilization 65.0% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Total Volumes 2027
PM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	56	122	124	882	638	49
Future Volume (Veh/h)	56	122	124	882	638	49
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	61	133	135	959	693	53
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type						
TWLTL TWLTL						
Median storage veh						
2 2						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1469	373	746			
vC1, stage 1 conf vol	720					
vC2, stage 2 conf vol	750					
vCu, unblocked vol	1469	373	746			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	79	79	84			
cM capacity (veh/h)	293	627	858			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	194	135	480	480	462	284
Volume Left	61	135	0	0	0	0
Volume Right	133	0	0	0	0	53
cSH	461	858	1700	1700	1700	1700
Volume to Capacity	0.42	0.16	0.28	0.28	0.27	0.17
Queue Length 95th (m)	16.4	4.5	0.0	0.0	0.0	0.0
Control Delay (s)	18.3	10.0	0.0	0.0	0.0	0.0
Lane LOS	C	A				
Approach Delay (s)	18.3	1.2			0.0	
Approach LOS	C					
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utilization			46.7%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Total Volumes 2027
 PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	45	8	258	80	22	113
Future Volume (Veh/h)	45	8	258	80	22	113
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	9	280	87	24	123
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	494	324			367	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	494	324			367	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	91	99			98	
cM capacity (veh/h)	527	722			1203	
Direction, Lane #						
	WB 1	NB 1	SB 1			
Volume Total	58	367	147			
Volume Left	49	0	24			
Volume Right	9	87	0			
cSH	550	1700	1203			
Volume to Capacity	0.11	0.22	0.02			
Queue Length 95th (m)	2.8	0.0	0.5			
Control Delay (s)	12.3	0.0	1.5			
Lane LOS	B		A			
Approach Delay (s)	12.3	0.0	1.5			
Approach LOS	B					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			34.7%	ICU Level of Service		A
Analysis Period (min)			15			

Appendix I

Synchro Reports

Total Volumes 2032

HCM Signalized Intersection Analysis
1: Maitland Dr & Hwy 62

Total Volumes 2032
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	36	86	139	168	109	63	41	492	72	59	875	28
Future Volume (vph)	36	86	139	168	109	63	41	492	72	59	875	28
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0		0.0	60.0		0.0	45.0		50.0	45.0		50.0
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (m)	100.0			65.0			0.0			0.0		
Satd. Flow (prot)	1752	1651	0	1719	1661	0	1805	3223	1455	1656	3471	1538
Flt Permitted	0.618			0.539			0.193			0.399		
Satd. Flow (perm)	1140	1651	0	975	1661	0	367	3223	1455	696	3471	1538
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		81			29				89			89
Link Speed (k/h)		50			50			60			60	
Link Distance (m)		927.0			367.9			238.3			786.0	
Travel Time (s)		66.7			26.5			14.3			47.2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	5%	4%	5%	3%	17%	0%	12%	11%	9%	4%	5%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	39	244	0	183	186	0	45	535	78	64	951	30
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes			Yes	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		55.4	47.8	47.8	56.6	48.4	48.4
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.50	0.43	0.43	0.51	0.44	0.44
v/c Ratio	0.10	0.39		0.54	0.31		0.16	0.38	0.11	0.15	0.62	0.04
Control Delay	25.4	19.9		36.4	23.9		13.2	22.9	4.0	21.6	44.5	8.5
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	25.4	19.9		36.4	23.9		13.2	22.9	4.0	21.6	44.5	8.5

HCM Signalized Intersection Analysis
 1: Maitland Dr & Hwy 62

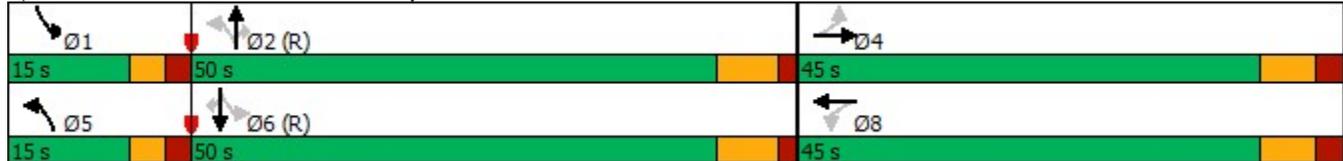
Total Volumes 2032
 AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	C	B		D	C		B	C	A	C	D	A
Approach Delay		20.7			30.1			20.0			42.1	
Approach LOS		C			C			C			D	
Queue Length 50th (m)	5.9	26.8		33.1	25.5		4.5	43.6	0.0	13.2	118.3	0.4
Queue Length 95th (m)	14.0	49.2		57.3	44.5		10.0	60.4	7.8	m20.5	140.4	m2.7
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	393	623		336	592		322	1401	682	454	1527	726
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.39		0.54	0.31		0.14	0.38	0.11	0.14	0.62	0.04

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.62
 Intersection Signal Delay: 31.5 Intersection LOS: C
 Intersection Capacity Utilization 73.8% ICU Level of Service D
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Analysis
9: Hwy 62 & Vermilyea Rd/Farnham Rd

Total Volumes 2032
AM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	67	43	69	13	34	66	16	337	23	121	856	125
Future Volume (vph)	67	43	69	13	34	66	16	337	23	121	856	125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	80.0			100.0			7.5			100.0		
Satd. Flow (prot)	1583	1712	1495	1421	1776	1442	1570	3167	1380	1656	3505	1495
Flt Permitted	0.733			0.726			0.179			0.524		
Satd. Flow (perm)	1222	1712	1495	1086	1776	1442	296	3167	1380	913	3505	1495
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			92			91			126
Link Speed (k/h)		50			50			80			50	
Link Distance (m)		249.3			317.7			872.4			250.2	
Travel Time (s)		17.9			22.9			39.3			18.0	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	14%	11%	8%	27%	7%	12%	15%	14%	17%	9%	3%	8%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	73	47	75	14	37	72	17	366	25	132	930	136
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6			3.6	
Link Offset(m)		0.0			0.0			0.0			0.0	
Crosswalk Width(m)		4.8			4.8			4.8			4.8	
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effct Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.17	0.08	0.13	0.04	0.06	0.13	0.06	0.30	0.04	0.25	0.68	0.21
Control Delay	26.7	25.0	4.1	24.5	24.7	3.7	6.8	11.8	0.1	13.9	31.2	5.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.7	25.0	4.1	24.5	24.7	3.7	6.8	11.8	0.1	13.9	31.2	5.5
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		17.6			12.4			10.9			26.4	
Approach LOS		B			B			B			C	
Queue Length 50th (m)	11.4	7.1	0.0	2.1	5.6	0.0	0.8	10.3	0.0	14.0	90.8	1.4

HCM Signalized Intersection Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

Total Volumes 2032
 AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (m)	22.9	15.8	7.3	6.8	13.1	6.8	m2.2	14.1	0.0	24.3	114.1	13.8
Internal Link Dist (m)		225.3			293.7			848.4			226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	419	588	574	373	610	555	263	1229	591	524	1360	657
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.08	0.13	0.04	0.06	0.13	0.06	0.30	0.04	0.25	0.68	0.21

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 21.3
 Intersection LOS: C
 Intersection Capacity Utilization 66.7%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Total Volumes 2032
AM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations				 	 	
Traffic Volume (veh/h)	31	82	87	449	877	42
Future Volume (Veh/h)	31	82	87	449	877	42
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	89	95	488	953	46
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1410	500	999			
vC1, stage 1 conf vol	976					
vC2, stage 2 conf vol	434					
vCu, unblocked vol	1410	500	999			
tC, single (s)	7.1	7.2	4.2			
tC, 2 stage (s)	6.1					
tF (s)	3.6	3.4	2.2			
p0 queue free %	87	82	86			
cM capacity (veh/h)	265	486	683			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	123	95	244	244	635	364
Volume Left	34	95	0	0	0	0
Volume Right	89	0	0	0	0	46
cSH	395	683	1700	1700	1700	1700
Volume to Capacity	0.31	0.14	0.14	0.14	0.37	0.21
Queue Length 95th (m)	10.5	3.9	0.0	0.0	0.0	0.0
Control Delay (s)	18.2	11.1	0.0	0.0	0.0	0.0
Lane LOS	C	B				
Approach Delay (s)	18.2	1.8	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			1.9			
Intersection Capacity Utilization			47.2%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Total Volumes 2032
 AM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	90	17	93	37	11	148
Future Volume (Veh/h)	90	17	93	37	11	148
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	98	18	101	40	12	161
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	306	121			141	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	306	121			141	
tC, single (s)	6.4	6.2			4.2	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.3	
p0 queue free %	86	98			99	
cM capacity (veh/h)	684	936			1383	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	116	141	173			
Volume Left	98	0	12			
Volume Right	18	40	0			
cSH	714	1700	1383			
Volume to Capacity	0.16	0.08	0.01			
Queue Length 95th (m)	4.6	0.0	0.2			
Control Delay (s)	11.0	0.0	0.6			
Lane LOS	B		A			
Approach Delay (s)	11.0	0.0	0.6			
Approach LOS	B					
Intersection Summary						
Average Delay			3.2			
Intersection Capacity Utilization			29.5%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Signalized Intersection Analysis
1: Maitland Dr & Hwy 62

Total Volumes 2032
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	61	118	68	193	95	77	120	942	128	53	732	46
Future Volume (vph)	61	118	68	193	95	77	120	942	128	53	732	46
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	60.0		0.0	60.0		0.0	45.0		50.0	45.0		50.0
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (m)	100.0			65.0			0.0			0.0		
Satd. Flow (prot)	1770	1760	0	1736	1679	0	1787	3539	1553	1556	3471	1568
Flt Permitted	0.617			0.596			0.233			0.168		
Satd. Flow (perm)	1149	1760	0	1089	1679	0	438	3539	1553	275	3471	1568
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		29			41				101			89
Link Speed (k/h)		50			50			60				60
Link Distance (m)		927.0			367.9			238.3				786.0
Travel Time (s)		66.7			26.5			14.3				47.2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	4%	6%	5%	1%	2%	4%	16%	4%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	66	202	0	210	187	0	130	1024	139	58	796	50
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6				3.6
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		4.8			4.8			4.8				4.8
Two way Left Turn Lane								Yes				Yes
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA		Perm	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2		2	6		6
Detector Phase	4	4		8	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		7.0	20.0	20.0	7.0	20.0	20.0
Minimum Split (s)	29.0	29.0		29.0	29.0		12.0	31.0	31.0	12.0	31.0	31.0
Total Split (s)	45.0	45.0		45.0	45.0		15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%		40.9%	40.9%		13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	4.5	4.5		4.5	4.5		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.5	2.5		2.5	2.5		2.0	1.7	1.7	2.0	1.7	1.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.0	7.0		7.0	7.0		5.0	6.7	6.7	5.0	6.7	6.7
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	Max	Max		Max	Max		None	C-Max	C-Max	None	C-Max	C-Max
Act Effct Green (s)	38.0	38.0		38.0	38.0		57.1	47.9	47.9	53.9	44.4	44.4
Actuated g/C Ratio	0.35	0.35		0.35	0.35		0.52	0.44	0.44	0.49	0.40	0.40
v/c Ratio	0.17	0.32		0.56	0.31		0.39	0.66	0.19	0.26	0.57	0.07
Control Delay	26.5	24.2		36.1	22.0		15.9	28.2	7.7	23.0	42.1	12.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.5	24.2		36.1	22.0		15.9	28.2	7.7	23.0	42.1	12.0

HCM Signalized Intersection Analysis
 1: Maitland Dr & Hwy 62

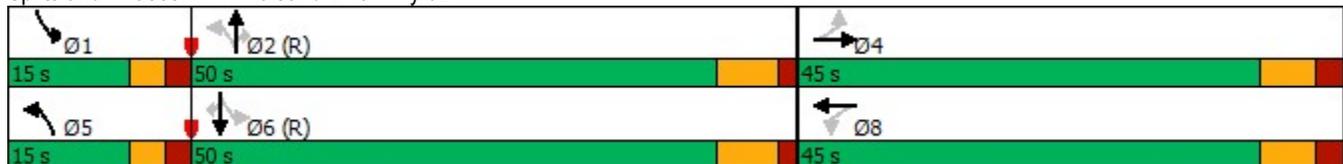
Total Volumes 2032
 PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
LOS	C	C		D	C		B	C	A	C	D	B
Approach Delay		24.8			29.5			24.7			39.2	
Approach LOS		C			C			C			D	
Queue Length 50th (m)	10.3	28.2		38.2	23.5		13.7	97.8	5.0	10.0	92.6	1.9
Queue Length 95th (m)	21.2	47.9		63.8	42.3		23.7	126.6	18.0	20.8	113.8	10.4
Internal Link Dist (m)		903.0			343.9			214.3			762.0	
Turn Bay Length (m)	60.0			60.0			45.0		50.0	45.0		50.0
Base Capacity (vph)	396	626		376	606		351	1540	733	256	1399	685
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.32		0.56	0.31		0.37	0.66	0.19	0.23	0.57	0.07

Intersection Summary

Area Type:	Other
Cycle Length:	110
Actuated Cycle Length:	110
Offset:	79.2 (72%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	75
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.66
Intersection Signal Delay:	30.0
Intersection LOS:	C
Intersection Capacity Utilization	74.3%
ICU Level of Service	D
Analysis Period (min)	15

Splits and Phases: 1: Maitland Dr & Hwy 62



HCM Signalized Intersection Analysis
9: Hwy 62 & Vermilyea Rd/Farnham Rd

Total Volumes 2032
PM Peak Hour

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	154	36	58	33	55	168	55	854	24	52	533	77
Future Volume (vph)	154	36	58	33	55	168	55	854	24	52	533	77
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (m)	80.0			100.0			7.5			100.0		
Satd. Flow (prot)	1787	1845	1524	1570	1900	1568	1687	3539	1346	1687	3406	1404
Flt Permitted	0.718			0.732			0.369			0.180		
Satd. Flow (perm)	1351	1845	1524	1209	1900	1568	655	3539	1346	320	3406	1404
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			183			91			91
Link Speed (k/h)		50			50			80				50
Link Distance (m)		249.3			317.7			872.4				250.2
Travel Time (s)		17.9			22.9			39.3				18.0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	1%	3%	6%	15%	0%	3%	7%	2%	20%	7%	6%	15%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	167	39	63	36	60	183	60	928	26	57	579	84
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)		3.6			3.6			3.6				3.6
Link Offset(m)		0.0			0.0			0.0				0.0
Crosswalk Width(m)		4.8			4.8			4.8				4.8
Two way Left Turn Lane								Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Turn Type	Perm	NA	Perm	Perm	NA	Perm	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4		4	8		8	2		2	6		6
Minimum Split (s)	22.2	22.2	22.2	22.2	22.2	22.2	12.0	27.3	27.3	12.0	27.3	27.3
Total Split (s)	45.0	45.0	45.0	45.0	45.0	45.0	15.0	50.0	50.0	15.0	50.0	50.0
Total Split (%)	40.9%	40.9%	40.9%	40.9%	40.9%	40.9%	13.6%	45.5%	45.5%	13.6%	45.5%	45.5%
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.9	5.9	3.0	5.9	5.9
All-Red Time (s)	2.2	2.2	2.2	2.2	2.2	2.2	2.0	1.4	1.4	2.0	1.4	1.4
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2	7.2	7.2	7.2	7.2	5.0	7.3	7.3	5.0	7.3	7.3
Lead/Lag							Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?							Yes	Yes	Yes	Yes	Yes	Yes
Act Effct Green (s)	37.8	37.8	37.8	37.8	37.8	37.8	55.0	42.7	42.7	55.0	42.7	42.7
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.50	0.39	0.39	0.50	0.39	0.39
v/c Ratio	0.36	0.06	0.11	0.09	0.09	0.28	0.14	0.68	0.04	0.20	0.44	0.14
Control Delay	29.8	24.7	2.6	25.3	25.1	4.9	5.7	13.1	0.1	13.7	26.1	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.8	24.7	2.6	25.3	25.1	4.9	5.7	13.1	0.1	13.7	26.1	4.6
LOS	C	C	A	C	C	A	A	B	A	B	C	A
Approach Delay		22.7			11.9			12.4			22.6	
Approach LOS		C			B			B			C	
Queue Length 50th (m)	28.1	5.9	0.0	5.5	9.1	0.0	2.1	21.9	0.0	5.8	50.0	0.0

HCM Signalized Intersection Analysis
 9: Hwy 62 & Vermilyea Rd/Farnham Rd

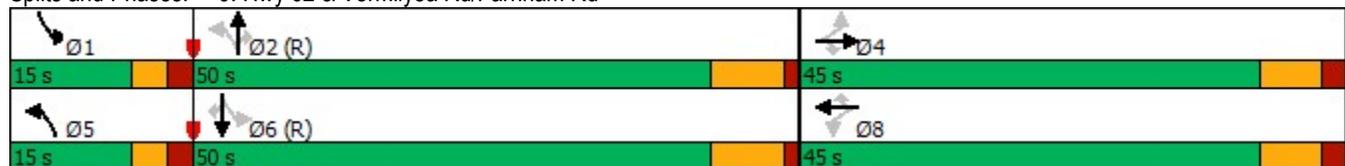
Total Volumes 2032
 PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 95th (m)	47.4	13.6	4.8	13.1	18.8	15.1	m3.6	26.2	m0.0	12.1	66.0	8.9
Internal Link Dist (m)	225.3				293.7		848.4				226.2	
Turn Bay Length (m)	90.0		90.0	130.0		60.0	100.0		100.0	60.0		60.0
Base Capacity (vph)	464	634	584	415	652	658	421	1373	578	284	1322	600
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.36	0.06	0.11	0.09	0.09	0.28	0.14	0.68	0.04	0.20	0.44	0.14

Intersection Summary

Area Type: Other
 Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 45 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 65
 Control Type: Pretimed
 Maximum v/c Ratio: 0.68
 Intersection Signal Delay: 16.8
 Intersection LOS: B
 Intersection Capacity Utilization 66.7%
 ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Hwy 62 & Vermilyea Rd/Farnham Rd



HCM Unsignalized Intersection Capacity Analysis
7: Hwy 62 & Roy Blvd

Total Volumes 2032
PM Peak Hour

						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	59	130	130	950	687	51
Future Volume (Veh/h)	59	130	130	950	687	51
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	64	141	141	1033	747	55
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			TWLTL	TWLTL		
Median storage veh			2	2		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1573	401	802			
vC1, stage 1 conf vol	774					
vC2, stage 2 conf vol	798					
vCu, unblocked vol	1573	401	802			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	76	77	83			
cM capacity (veh/h)	270	602	817			
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2
Volume Total	205	141	516	516	498	304
Volume Left	64	141	0	0	0	0
Volume Right	141	0	0	0	0	55
cSH	435	817	1700	1700	1700	1700
Volume to Capacity	0.47	0.17	0.30	0.30	0.29	0.18
Queue Length 95th (m)	19.7	5.0	0.0	0.0	0.0	0.0
Control Delay (s)	20.5	10.3	0.0	0.0	0.0	0.0
Lane LOS	C	B				
Approach Delay (s)	20.5	1.2	0.0			
Approach LOS	C					
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utilization			49.1%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 11: Sidney St & Kempton Ave

Total Volumes 2032
 PM Peak Hour

						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	47	9	278	84	23	122
Future Volume (Veh/h)	47	9	278	84	23	122
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	51	10	302	91	25	133
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		None	
Median storage veh						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	530	348			393	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	530	348			393	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	90	99			98	
cM capacity (veh/h)	502	700			1177	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	61	393	158			
Volume Left	51	0	25			
Volume Right	10	91	0			
cSH	526	1700	1177			
Volume to Capacity	0.12	0.23	0.02			
Queue Length 95th (m)	3.1	0.0	0.5			
Control Delay (s)	12.7	0.0	1.4			
Lane LOS	B		A			
Approach Delay (s)	12.7	0.0	1.4			
Approach LOS	B					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization			36.0%		ICU Level of Service	A
Analysis Period (min)			15			

Appendix J

Left Turn Lane Warrant Assessment

Left Turn Warrant for Sidney St at Kempton Ave

Total Volumes, Peak Hours 2032

	AM	PM
V_o	130	362
V_A	159	145
V_L	11	23
$V_{\%L}$	5%(6.9%)	15%(15.9%)
	—	—

